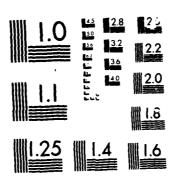
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DESIGN CALCULATIONS

93' MLW STRUCTURE

EAST COAST AIR COMBAT MANEUVERING RANGE
OFFSHORE KITTY HAWK, NORTH CAROLINA
CONTRACT NO. N62477-76-C-0179
MODIFICATION NO. P0001

REPORT NO. 27-771-95

PREPARED FOR
NAVAL FACILITIES ENGINEERING COMMAND
DEPARTMENT OF THE NAVY
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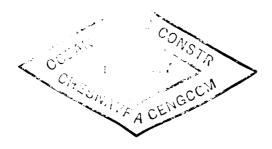
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4. PERFORMING ORGANIZATION REPORT NUMBER Report No. 27-771-95	5. MONITORING ORGANIZATION REPORT # FPO 7614
6a. NAME OF PERFORM. ORG. 6b. OFFICE SYM Crest Engineering, Inc.	7a. NAME OF MONITORING ORGANIZATION Ocean Engineering & Construction Project Office CHESNAVFACENGCOM
6c. ADDRESS (City, State, and Zip Code) Tulsa, OK	7b. ADDRESS (City, State, and Zip) BLDG. 212, Washington Navy Yard Washington, D.C. 20374-2121
8a. NAME OF FUNDING ORG. 8b. OFFICE SYM	9. PROCUREMENT INSTRUMENT INDENT # Contract No. N62477-76-C-0179 Modification No. P0001
8c. ADDRESS (City, State & Zip)	10. SOURCE OF FUNDING NUMBERS PROGRAM PROJECT TASK WORK UNIT ELEMENT # # ACCESS #
11. TITLE (Including Security Classificati Design Calculations 93'MLW Structure East Offshore Kitty Hawk, North Carolina 12. PERSONAL AUTHOR(S)	
13a. TYPE OF REPORT 13b. TIME COVERED FROM TO	14. DATE OF REP. (YYMMDD) 15. PAGES 76-09
16. SUPPLEMENTARY NOTATION	
FIELD GROUP SUB-GROUP Air Co	T TERMS (Continue on reverse if nec.) mbat Maneuvering Range, Towers, construction
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DESIGN CALCULATIONS 93' MLW PLATFORM

EAST COAST AIR COMBAT MANEUVERING RANGE
OFFSHORE KITTY HAWK, NORTH CAROLINA
CONTRACT NO. N62477-76-C-0179
MODIFICATION NO. P0001

Report No. 27-771-95

Prepared for

NAVY FACILITIES ENGINEERING COMMAND DEPARTMENT OF THE NAVY CHESAPEAKE DIVISION

Ву

CREST ENGINEERING, INC. TULSA, OKLAHOMA

September 1976

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SECTION 1.0

INTRODUCTION

1.1 PURPOSE OF REPORT

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The purpose of this report is to provide a document insuring the structural integrity of one of the four marine structures of the U.S. Navy East Coast Air Combat Maneuvering Range, offshore of Kitty Hawk, North Carolina. This structure is identified as Structure 2 at Site 2, resting in approximately 93 feet of water.

This report (No. 27-771-95) is part of the documentation required by U. S. Government Contract No. N62477-76-C-0179, Modification No. P0001, let by the Naval Facilities Engineering Command, Department of the Navy, Chesapeake Division with Crest Engineering, Inc., Tulsa, Oklahoma.

1.2 DESCRIPTION OF STRUCTURE

1.2.A Purpose of Structure

The structure, at Site 2 is part of a series of structures comprising the U.S. Navy East Coast Air Combat Maneuvering Range. Its purpose is to provide a platform to support electronic instrumentation necessary for the proper functioning of the East Coast Air Combat Maneuvering Range.

The equipment on the structure includes:

- Navigation and aircraft warning lights attached to the top
 of each of the three columns.
- 2. A signal horn attached to the underside of the Equipment

 Deck extending toward the sea.
- 3. A receiver-transmitter assembly attached to the Equipment Deck in the vicinity of the southwest corner.
- A solar panel assembly attached to the cantilevered deck on the south side of the Equipment Deck.
- A battery assembly fastened just north of the solar panel assembly.
- 6. One air-to-ground antenna attached in the center of the Upper Deck.

- One ground-to-ground antenna, eight feet in diameter, mounted on a vertical guide attached to the southwest column. This antenna is adjustable vertically from El. (+) 75.0' to El. (+) 25.0'.
- 8. A hand-operated, two-ton marine winch located on the Equipment Deck in the southeast corner.

1.2.B Location

The site for the East Coast Air Combat Maneuvering Range is approximately 26 miles offshore of Kitty Hawk, North Carolina.

Structure 2 will be erected within a half a mile of coordinates 13, 152, 623 North by 1, 566, 734 West $(N36^{\circ}\ 13'\ 36",\ W\ 75^{\circ}\ 14'\ 59")$ in 93 feet of water.

The structure will be oriented so that the side of the platform with the cantilevered solar panel deck will face due South.

This places the boat landing on the northeast side of the structure,
and locates the column with the ground-to-ground antenna nearest
the shore.

1.2.C Structural Description

The marine structure consists of a three-pile jacket (template) with equilaterally spaced legs through which steel piles are driven into the seabed. The jacket is then secured to the piling by welding shim plates in the annulus between the jacket leg and piling at the top of the jacket legs. A superstructure consisting of an equipment deck and an upper deck is then attached to the piling above the top of the jacket. This tripod structure has the following features:

- Upper Deck elevation is at (+) 75.0 feet above Mean Low
 Water to provide an adequate envelope for the hoist on the
 Equipment Deck.
- 2. Equipment Deck elevation is at (+) 60.0 feet above MLW to provide an air gap of 8.0 feet between the deck and the maximum crest of the 50 year storm.
- 3. To avoid any shadowing of the cells, a cantilevered deck is provided on the south side of the Equipment Deck to support the Solar Panel Assembly.
- 4. The only diagonal bracing framing the superstructure is between El. (+) 60.0 feet and El. (+) 45.0 feet.
- 5. The equilateral pile spacing at the pile cut-off El. (+) 16.5 feet is 29.0 feet from centerline to centerline.
- 6. The true jacket batter is 6 to 1 for each of the three legs.

- Horizontal bracings for the jacket are located at El.
 (+) 12.0, El. (-) 13.0, El. (-) 39.0, El. (-) 66.0 and El. (-) 93.0. In addition to the perimeter bracings, secondary horizontal bracings connecting the mid-points of the perimeter bracings are located at each of the above elevations. Diagonal bracing connect the levels.
- 8. A boat landing is provided on the northeast side of the structure from El. (+) 9.0 to El. (-) 3.0.
- 9. Boat fenders are attached to the two jacket legs on the boat landing side of the structure to protect the structure from sustaining damage from large impacts of approaching boats. The fenders consist of rubber tires installed around a vertical concrete filled pipe from El. (+) 12.0 to El. (-) 7.5.

1.3 DESIGN CRITERIA

1.3.A Purpose of Structure

1. Wave Data - 50 year storm

MLW Depth	93.0 ft.
Storm Wave Height	60.8 ft.
Storm Wave Period	13.6 sec.
Maximum Storm Tide	3.6 ft.
Maximum Astronomical Tide	4.5 ft.
Extreme Surface Current	4.5 ft./sec.
Mudline Current	0.8 ft./sec.

2. Wind Data

Maximum Gust	174.0 mph
1 Minute Wind	145.0 mph
1 Hour Wind	114.0 mph

The approach of the storm wind and wave can be from any direction.

1.3.B Foundation Criteria

 The basis for the foundation design is a McClelland report to Cubic Corporation entitled "Foundation Investigation East Coast ACMR Ocean Structures, Volume I". The soil information to be used in this analysis is one boring at Site 2. 2. Due to the nature of the sea bottom and sea bottom currents, scouring of 5 feet below the mudline will be used in the piling design to develop the theorectical soil resistance to laterally applied loads.

1.3.C Live Loads

DOMESTIC CONTRACTOR

The design live loads will be:

Equipment Deck

150 psf

Top Deck

100 psf

The loads will be distributed uniformly over the entire deck areas.

1.3.D Material

All structural shapes or fabricated tubular goods are to be ASTM A-36 or equal except for the material used for the structure legs at the joint cans which is to be ASTM A-633, Grade A.

1.3.E Corrosion Protection

- All portions of the platform above elevation (-) 4.0 feet will be painted.
- 2. All main structural members located within the splash zone will have an extra 1/2" of sacrificial steel added to their wall thickness. This can be in the form of extra wall thickness or a 1/2" steel plate wrap.

3. The portion of the platform below elevation (-) 4.0 feet will be protected by cathodic protection. This will be provided by sacrificial anodes having a theoretically expected life of twenty years.

1.3.F Pile-Jacket Connection

STATES OF THE PROPERTY OF

The platform is analyzed as if the annulus between the jacket and the piling is not grouted. Shim plates will be provided at each horizontal bracing level. Jacket to pile connection is made by welding at elevation (+) 16.5 feet.

1.3.G Design Standards

The criteria employed for determination of structural acceptability are specified by the following documents:

- 1. American Petroleum Institute (API):
 - RP 2A Recommended Practice for Planning,

 Designing and Constructing Fixed Offshore

 Platforms; 7th Edition, January 1976.
 - Spec. 2H Carbon Manganese Steel for Offshore

 Platform Tubular Joints; 1st Edition,

 January 1974; Supplement 1, January 1975.
- American Institute of Steel Construction (AISC):
 Specification for the Design, Fabrication
 and Erection of Structural Steel for Buildings; February 12, 1969.

3. American Society for Testing and Materials (ASTM):

A36-75 Structural Steel

CON SESSESS CHICAGO

A633-75 Normalized High-Strength Low-Alloy Structural Steel.

4. American Welding Society (AWS):

D1.1-75 Structural Welding Code. (Rev. 1-76)

1.4 DESIGN ASSUMPTIONS

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1.4.A Environmental Criteria

1. Wave Data

(a) Wave Coefficients

$$C_D = 0.74$$
 $C_M = 1.34$

These wave coefficients are the wave coefficients used to generate Dean's Stream Function wave grid profile for a 3.0 ft. diameter pile. It is assumed that these wave coefficients are applicable to all tubular shapes in this structure.

(b) Wave-Current Coupling The pressures indicated by Dean's Stream Function wave grid profile include the coupling of the wave forces with the current forces.

2. Wind Data

The structure is designed for the one minute wind superimposed on the 50 year storm wave.

1.4.B Equipment Loads

All equipment loads are included in the area live load. This is a valid assumption because no piece of equipment has a density to produce a load greater than 150 psf.

1.4.C Marine Growth

- A 1.0" marine growth allowance on the radius is included on all primary jacket members from (+) 0.0 ft. to the mudline.
- 2. The effective diameter for the drag area produced by the marine growth is:

Deff = (Dact + 2.0")
$$(\frac{1.02}{0.74})$$

where $1.02 = C_d$ for medium barnacle fouling $0.74 = Assumed magnitude of Dean's <math>C_d$

1.5 DESIGN SUMMARY

1.5.A Environmental Forces:

Total wind and wave shear force (Maximum - Load Condition #2)	1,360 kips
Total overturning moment	117,139 kips

1.5.B Pile Axial Loads:

2,914 kips
1,972 kips

1.5.C Structural Dimensions:

Piling

Outside Diameter	42 in.
Maximum Wall Thickness	2.37 in.
Minimum Wall Thickness	2.00 in.
Penetration Below Mudline	275 ft.

Jacket

Spacing at Mudline	60.0 ft.
Spacing at Work-Point Level	29.0 ft.
Height (Mudline to Work-Point)	109.5 ft.

Superstructure

Equipment Deck Area	556.0 ft. ²
Top Deck Area	364.0 ft. ²

1.5.D Structural Steel Weight

<u>Item</u>	Structure
Piling	1,043 kips
Superstructure	132 kips
Jacket	353 kips
Boat Landing	24 kips
Boat Fenders	14 kips
Anodes	16 kips
Total	1,581 kips

SECTION 2.0 STRUCTURAL DRAWINGS

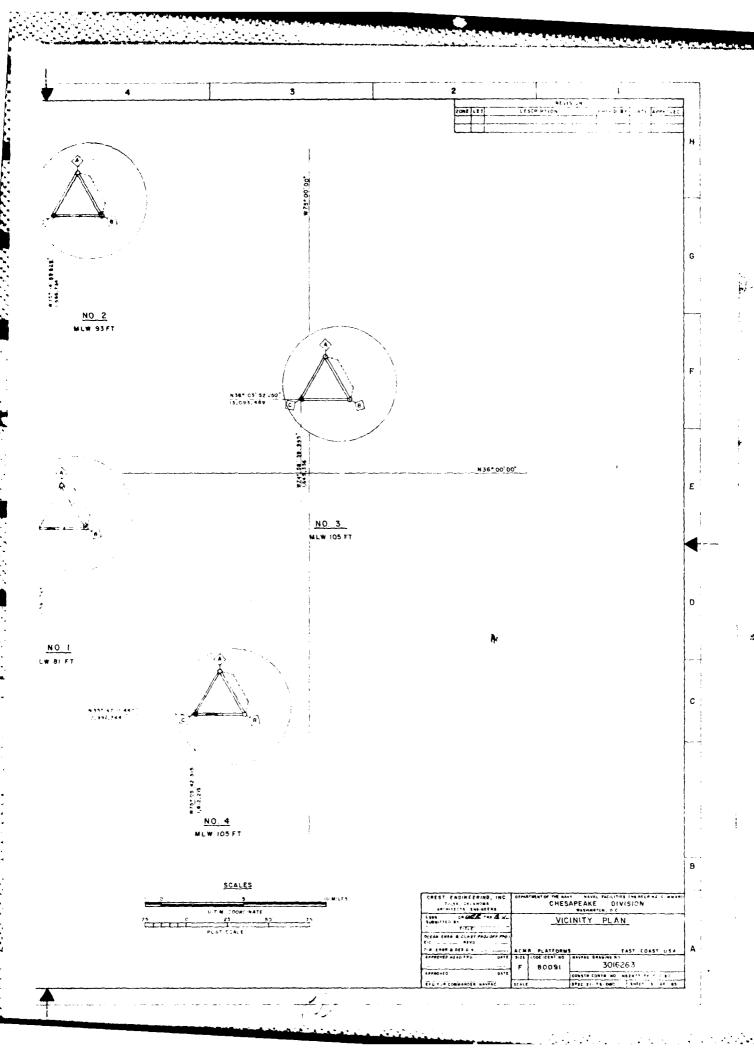
2.1 INTRODUCTION

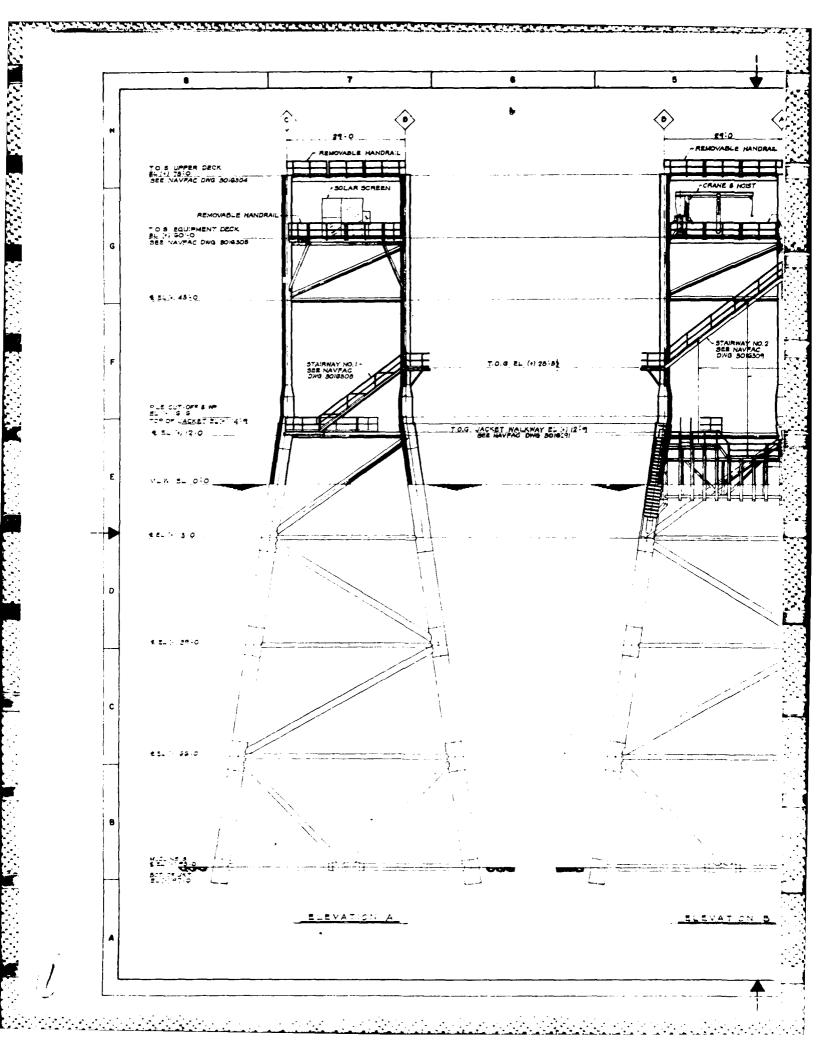
A few selected structural engineering drawings are included in this chapter for reference to the design calculations. The Introduction to each Section in this report lists the appropriate drawings pertinent to that particular section. Reference then can be made to this section of the report for a reduced copy of the drawing.

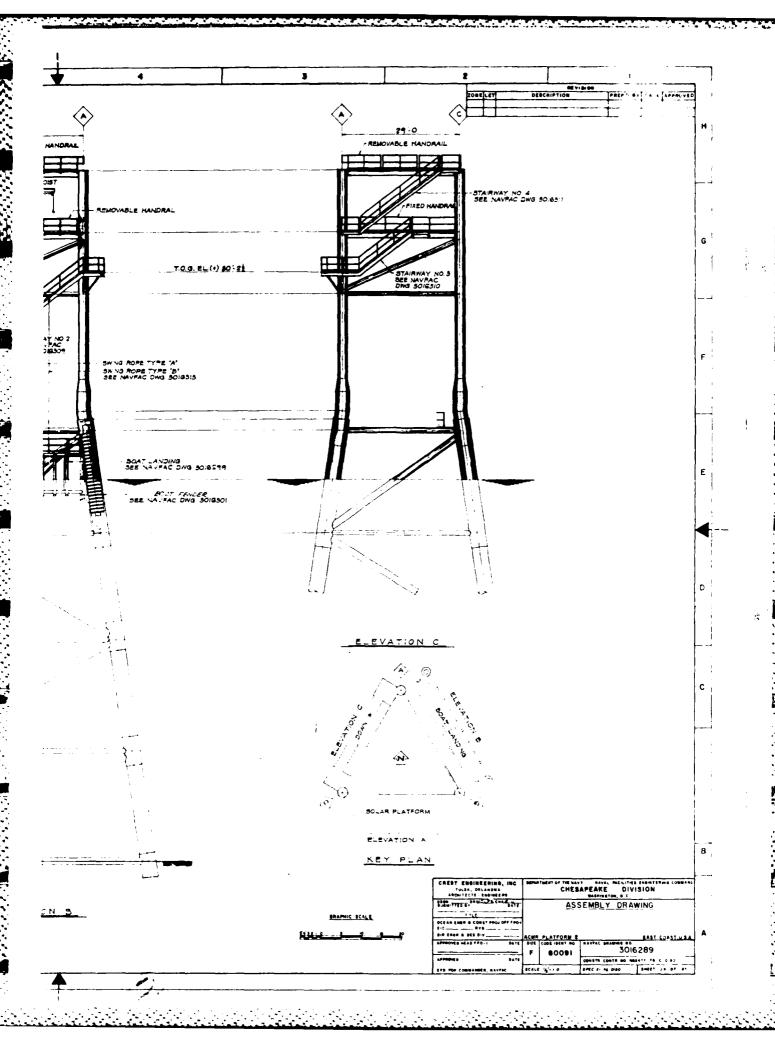
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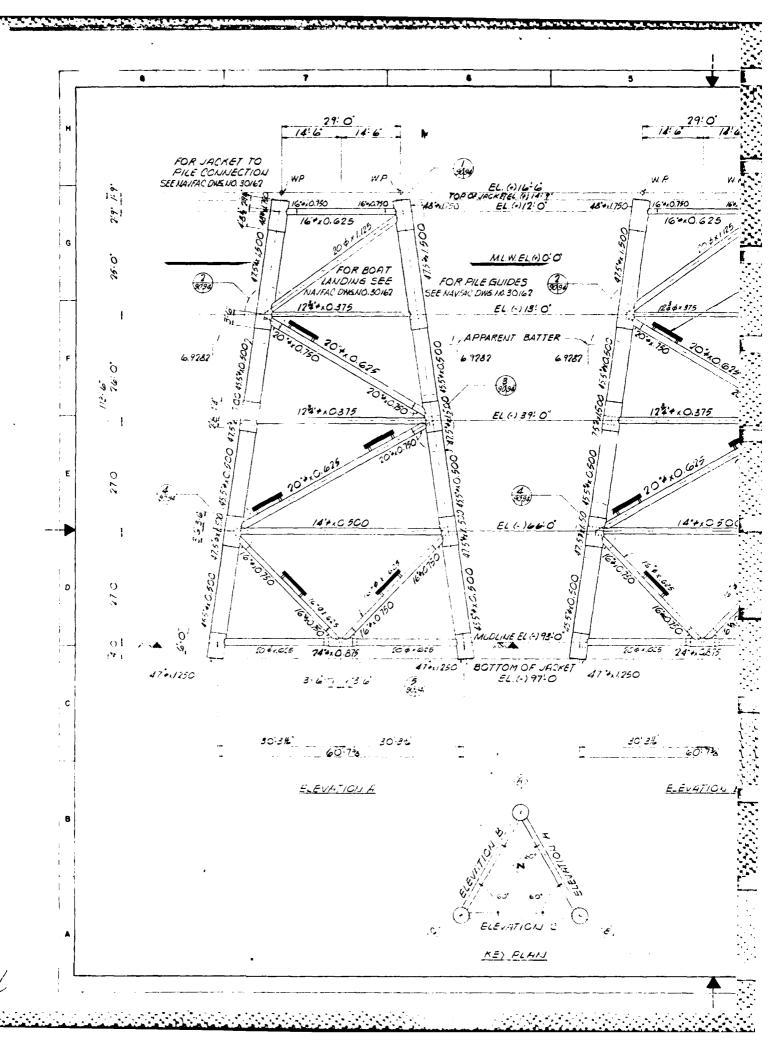
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3016291	Jacket - Plan at El. (+) 12'-0"	2.05
3016292	Jacket - Plan at El. (-) 13'-0" & (-) 39'-0"	2.06
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3016303	Superstructure - Elevations	2.13
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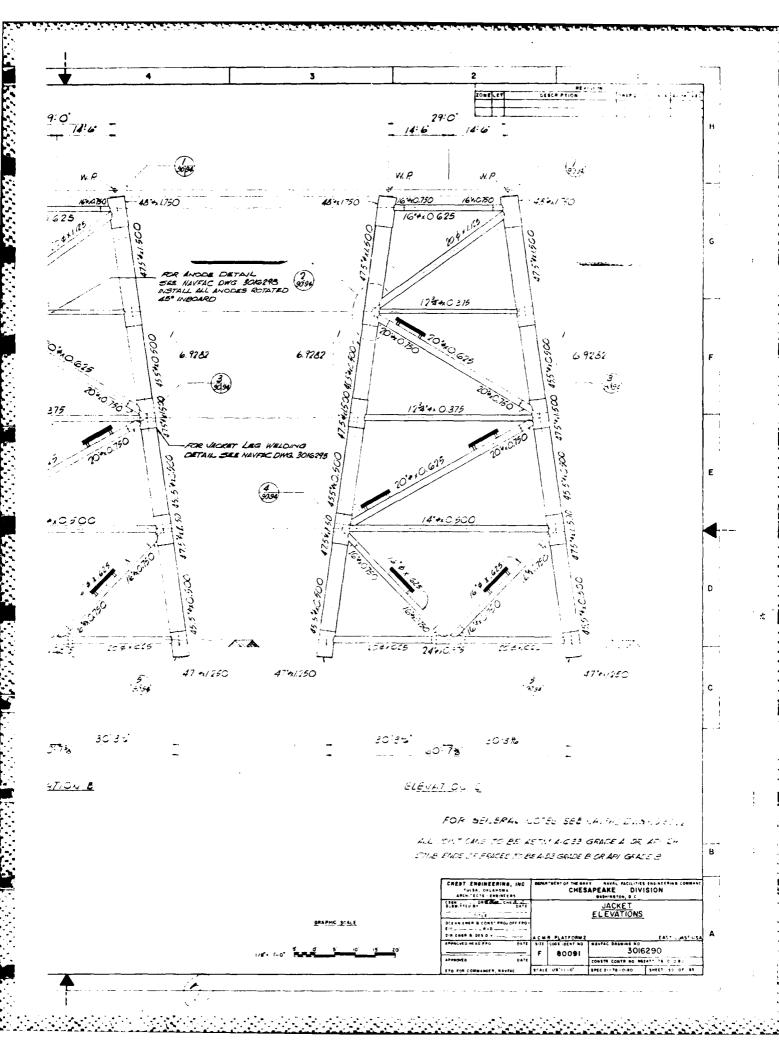
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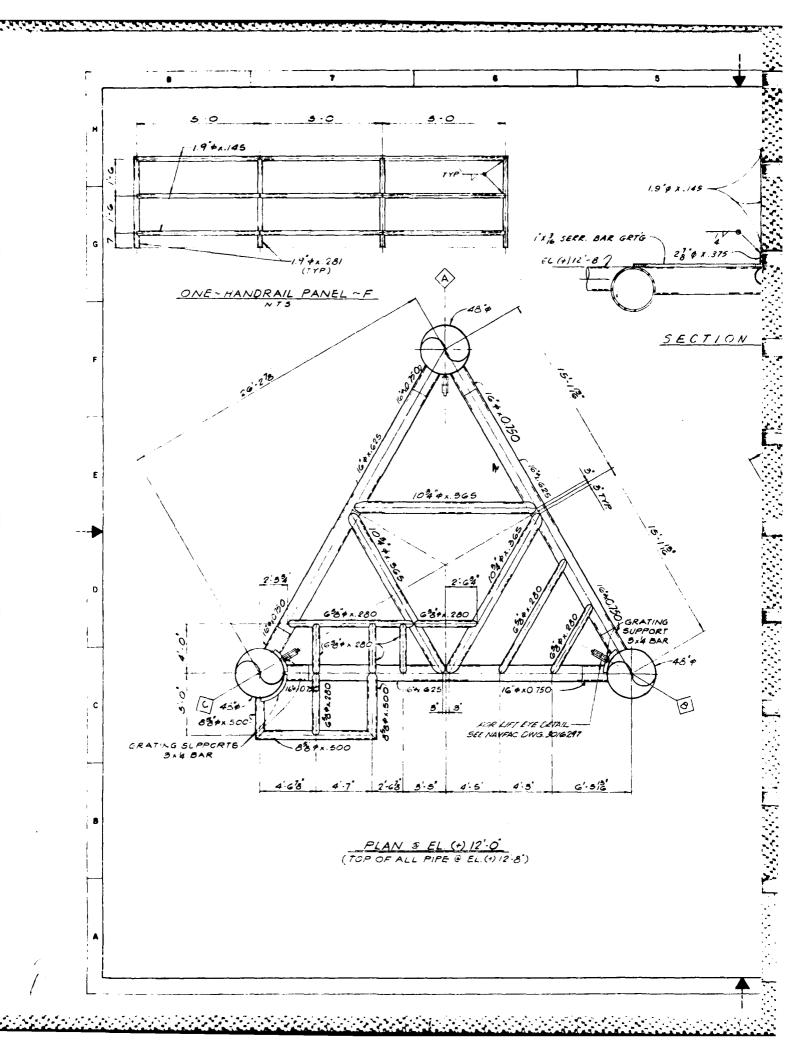


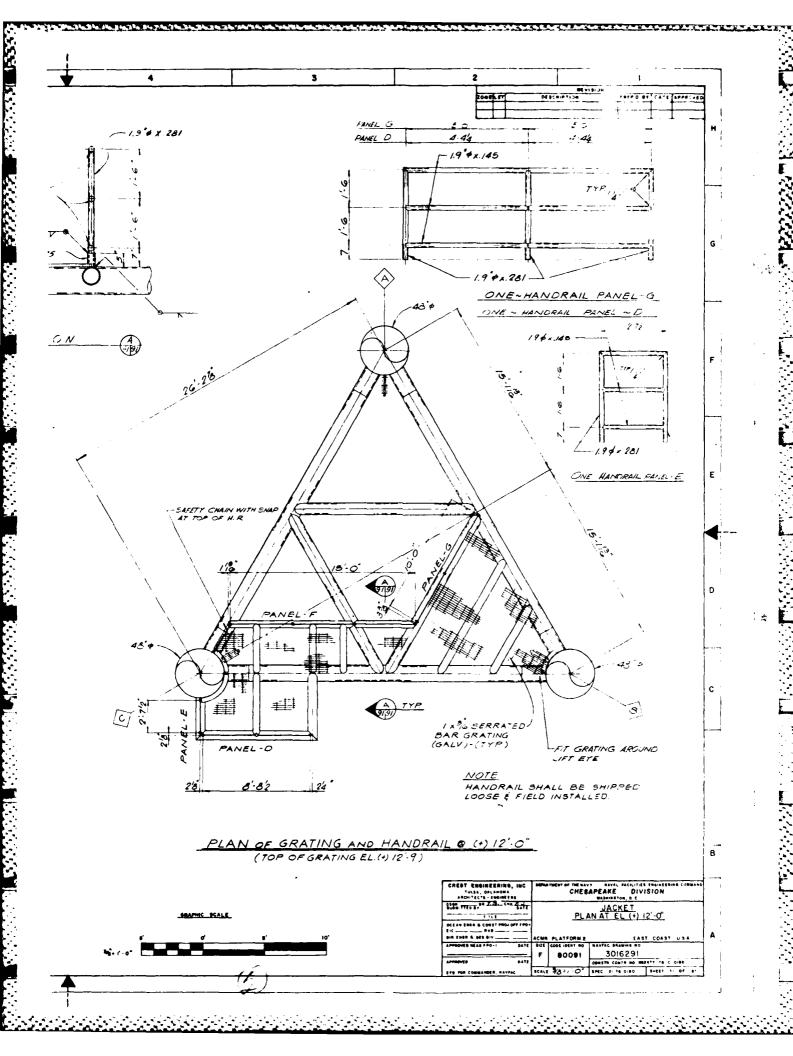


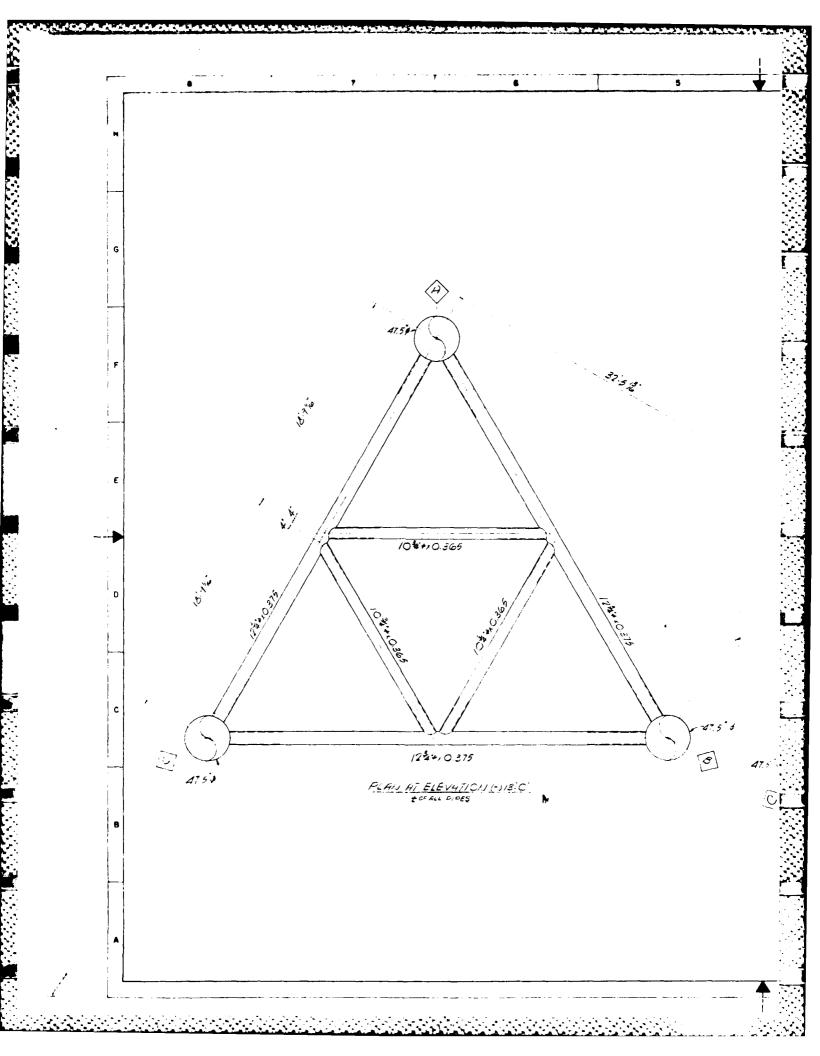


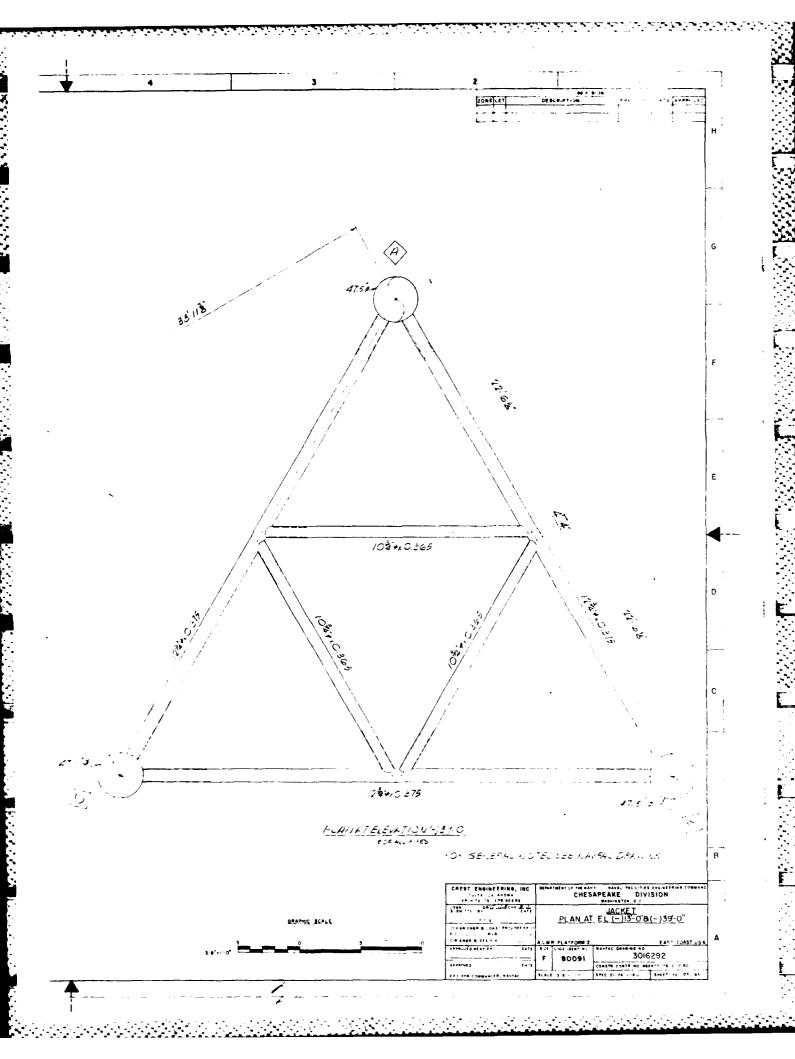


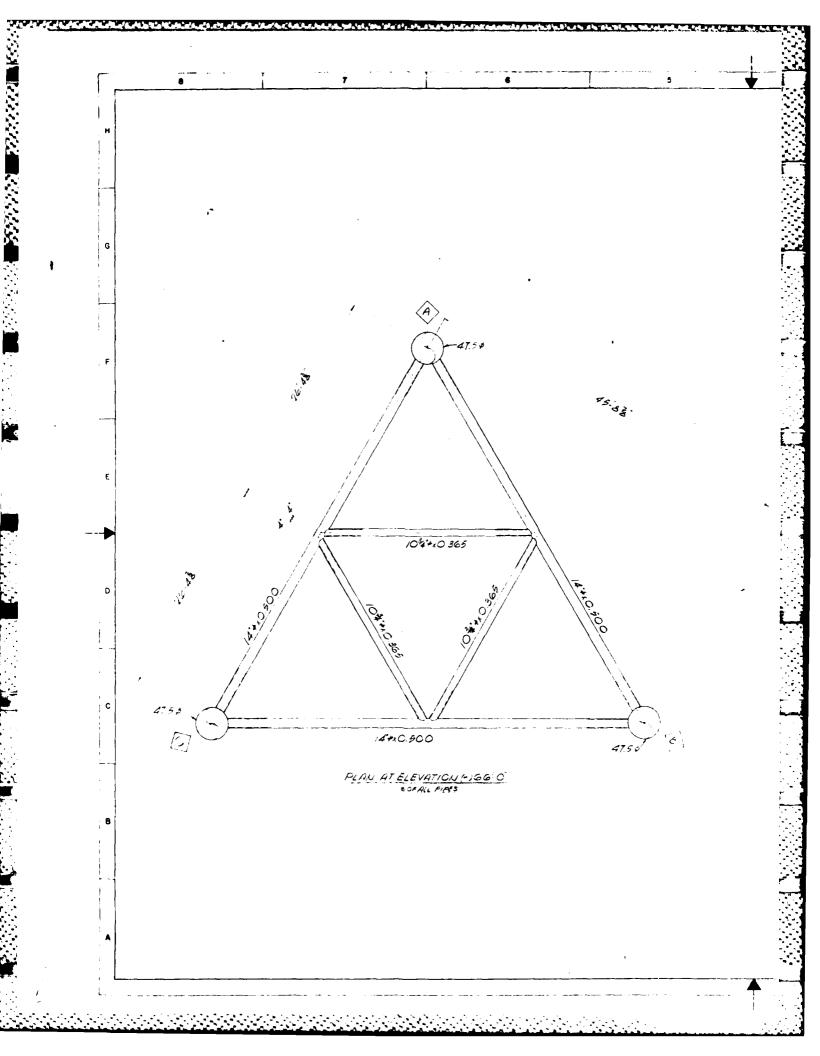


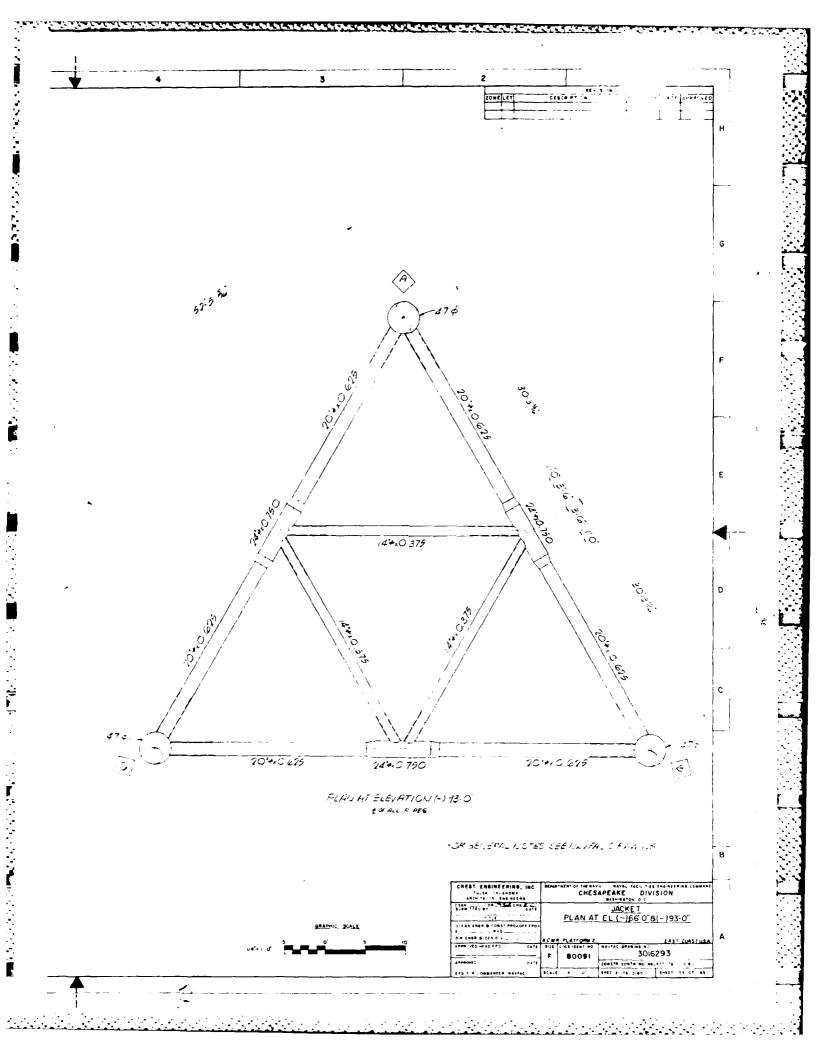


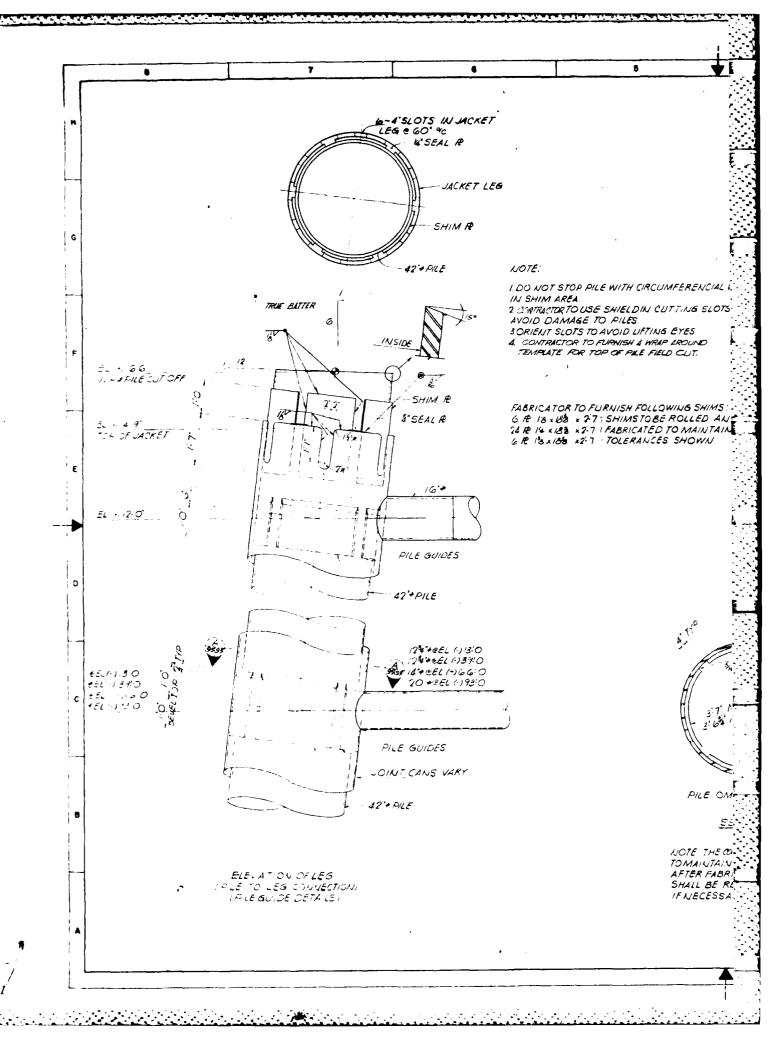


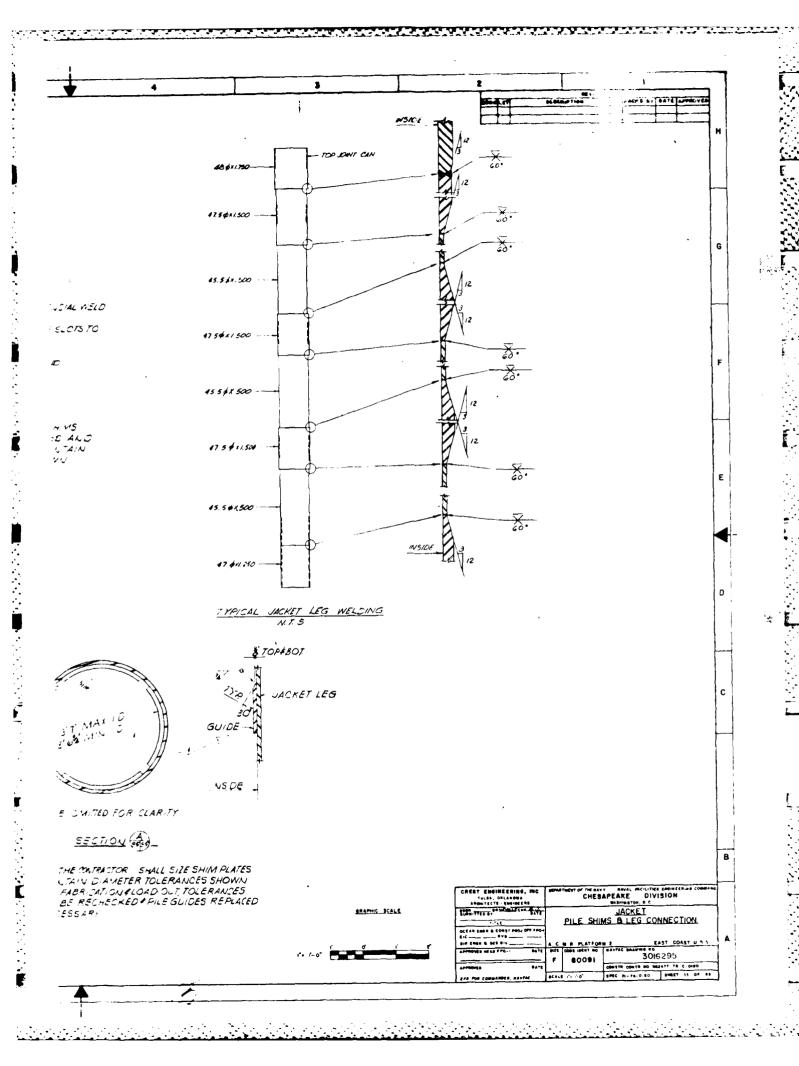


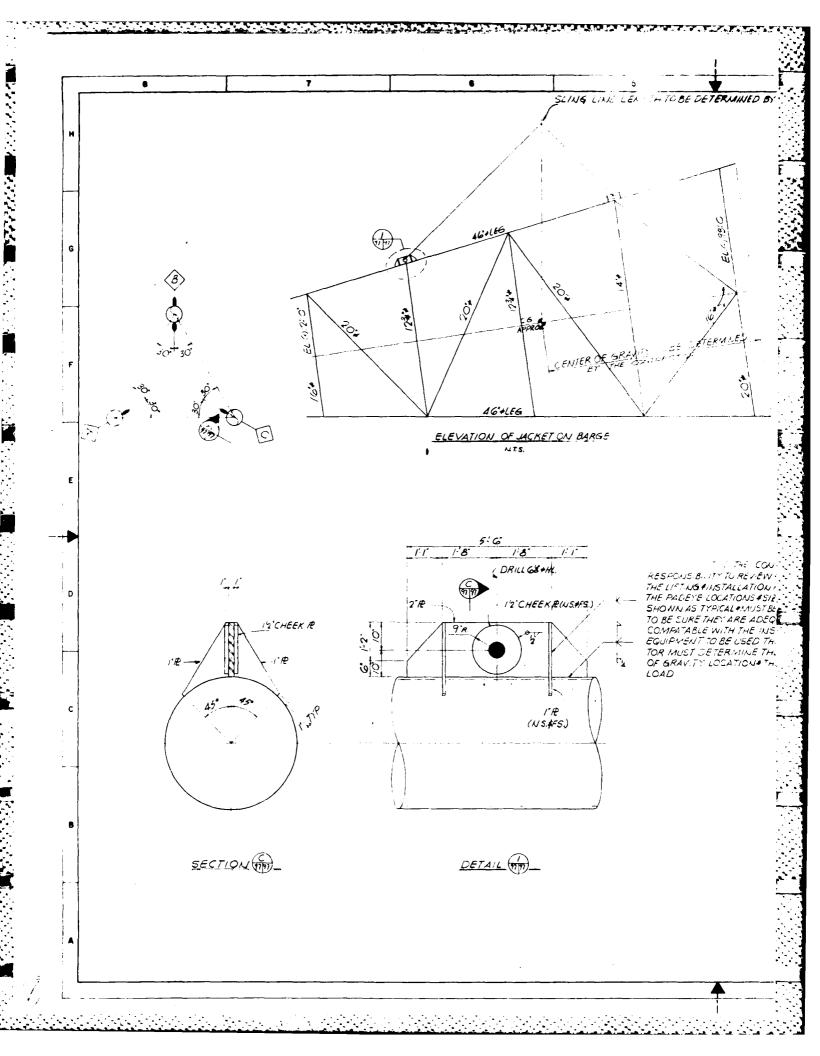


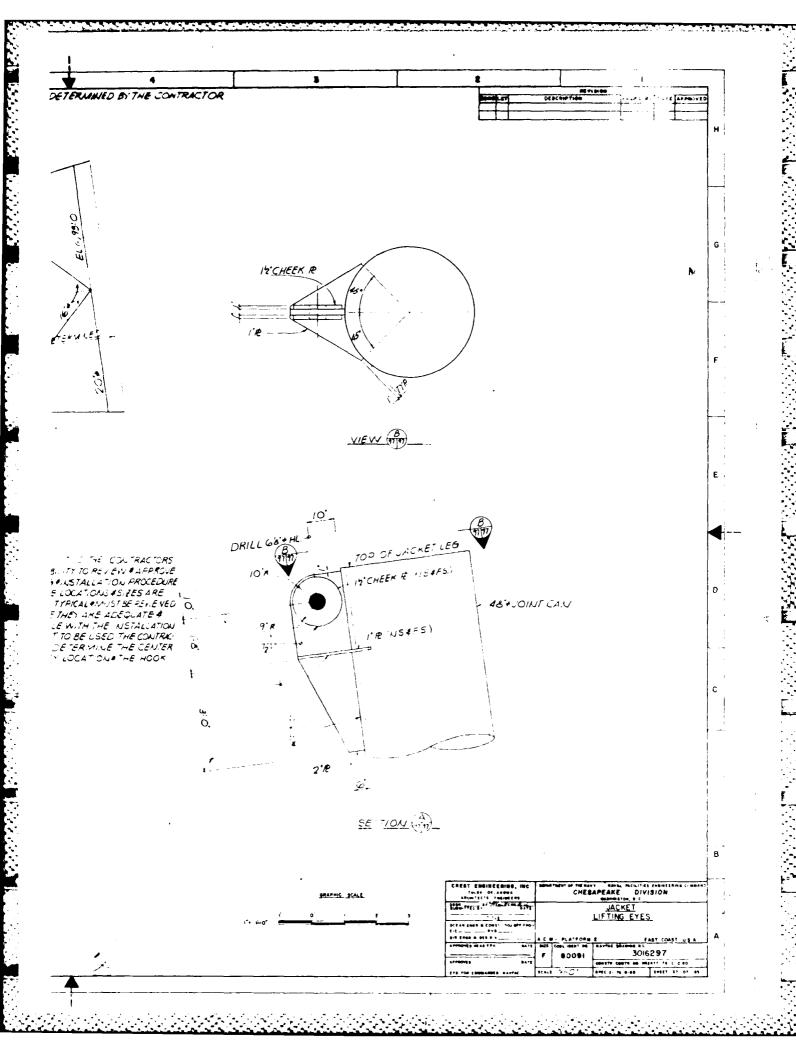


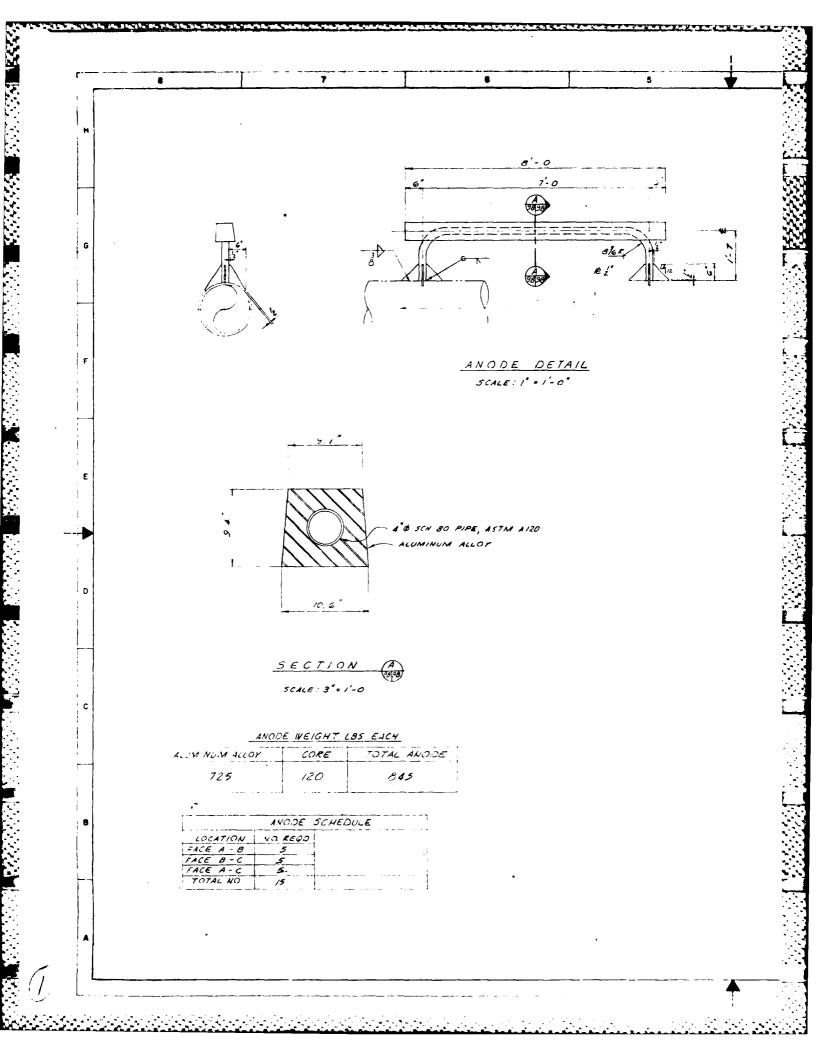


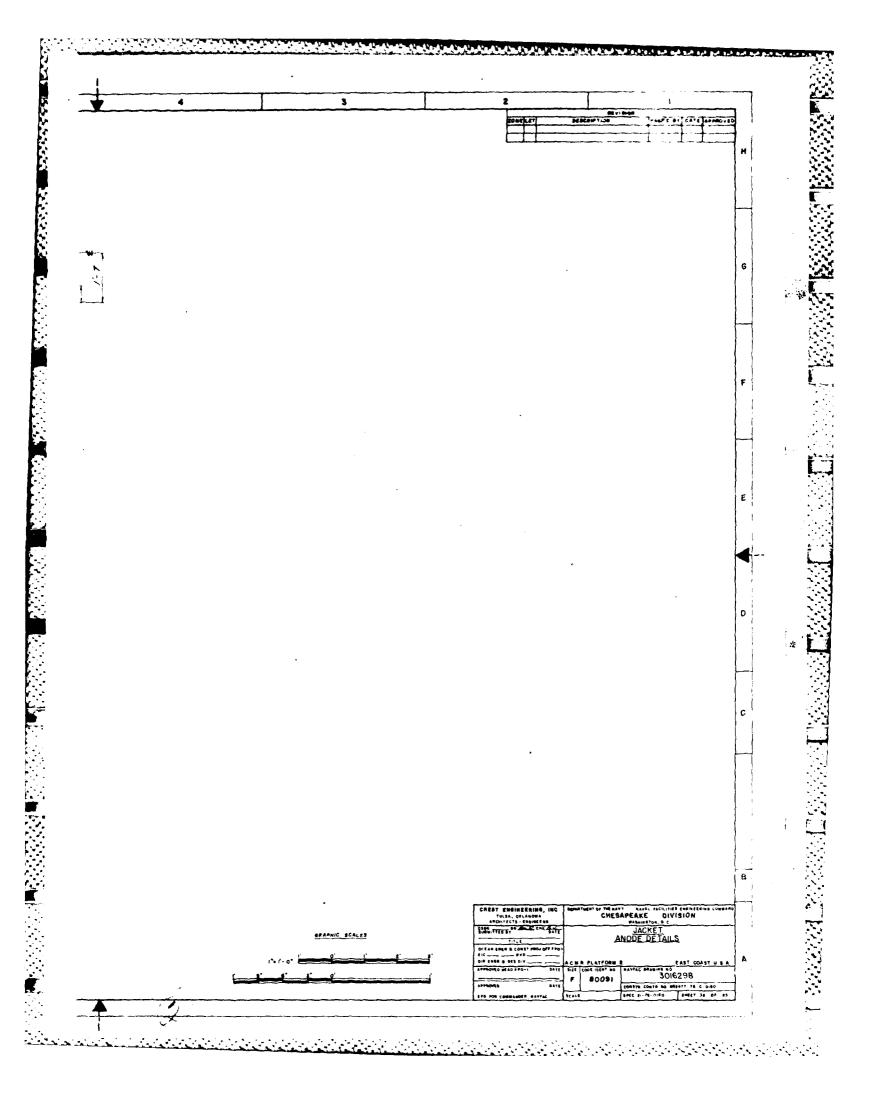


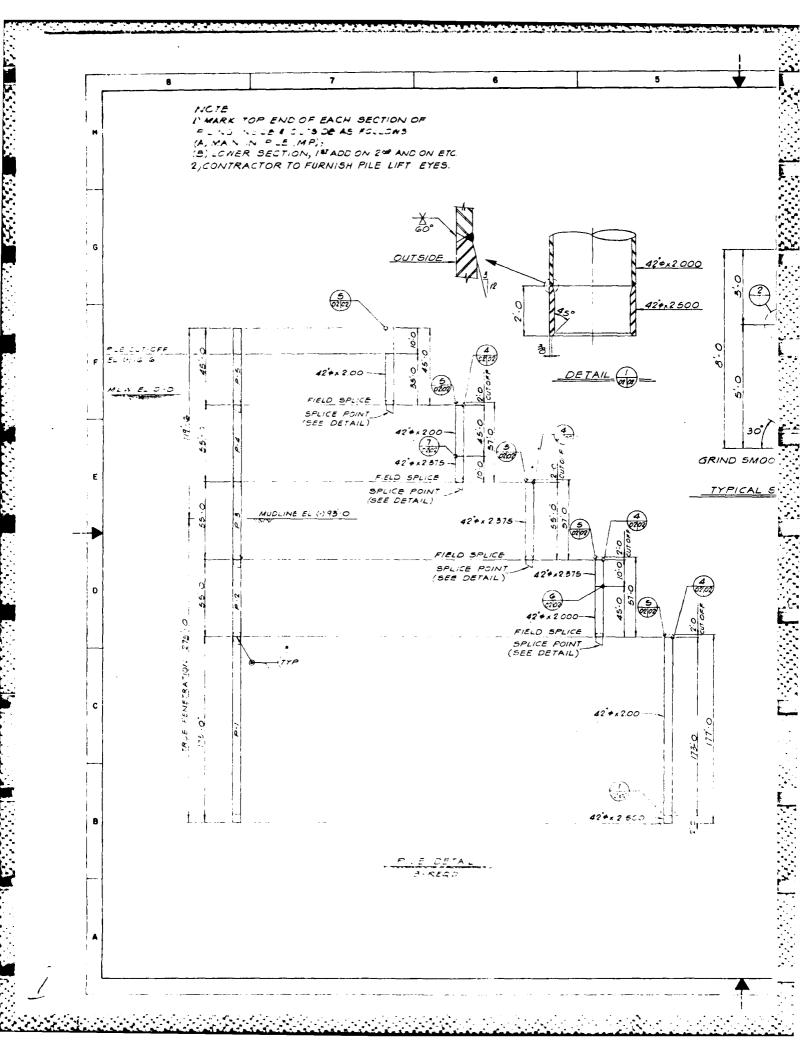


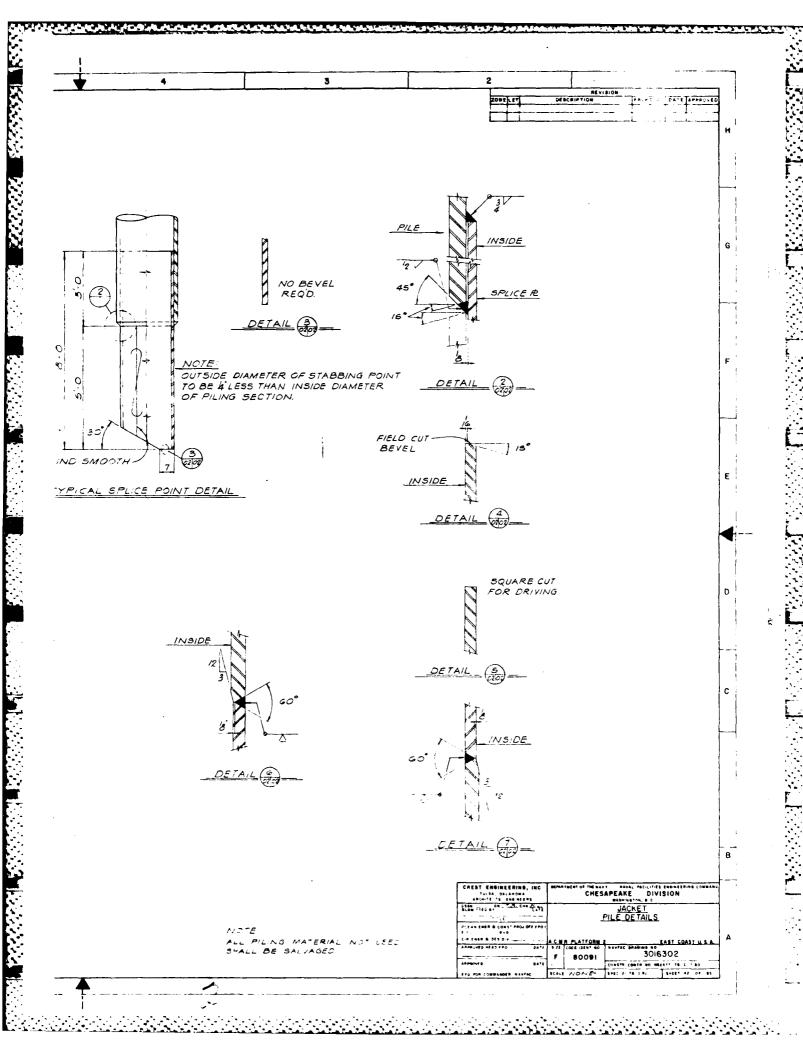


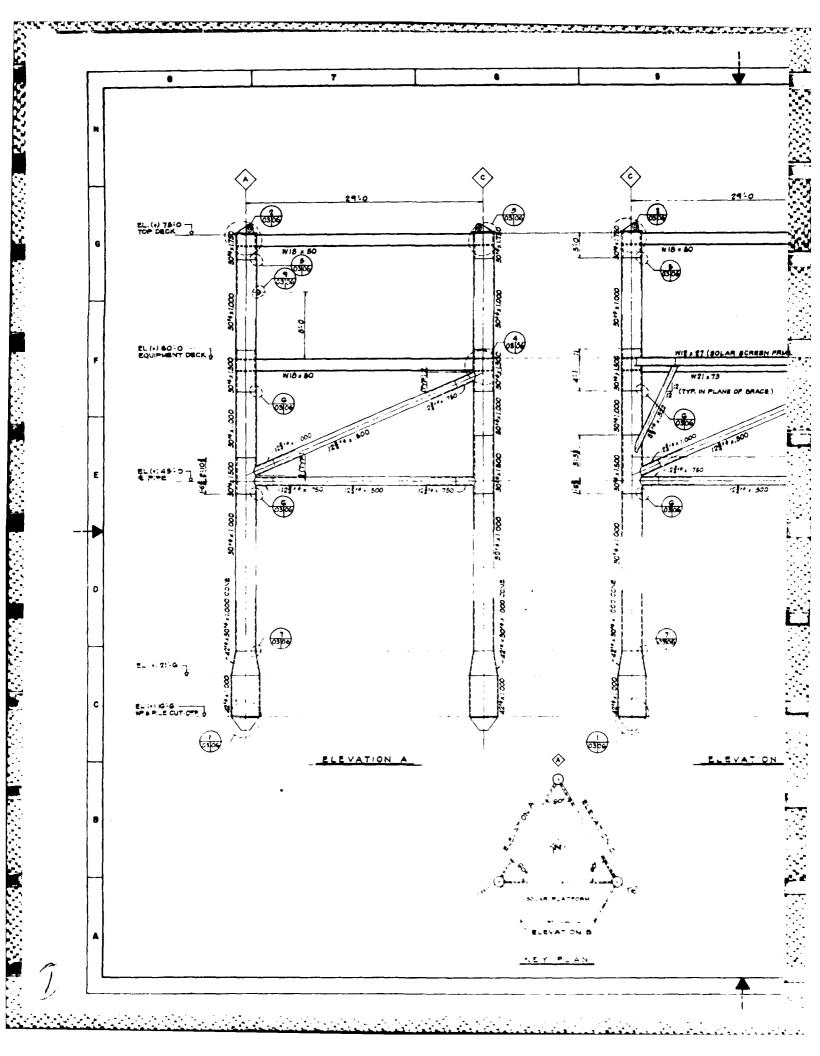


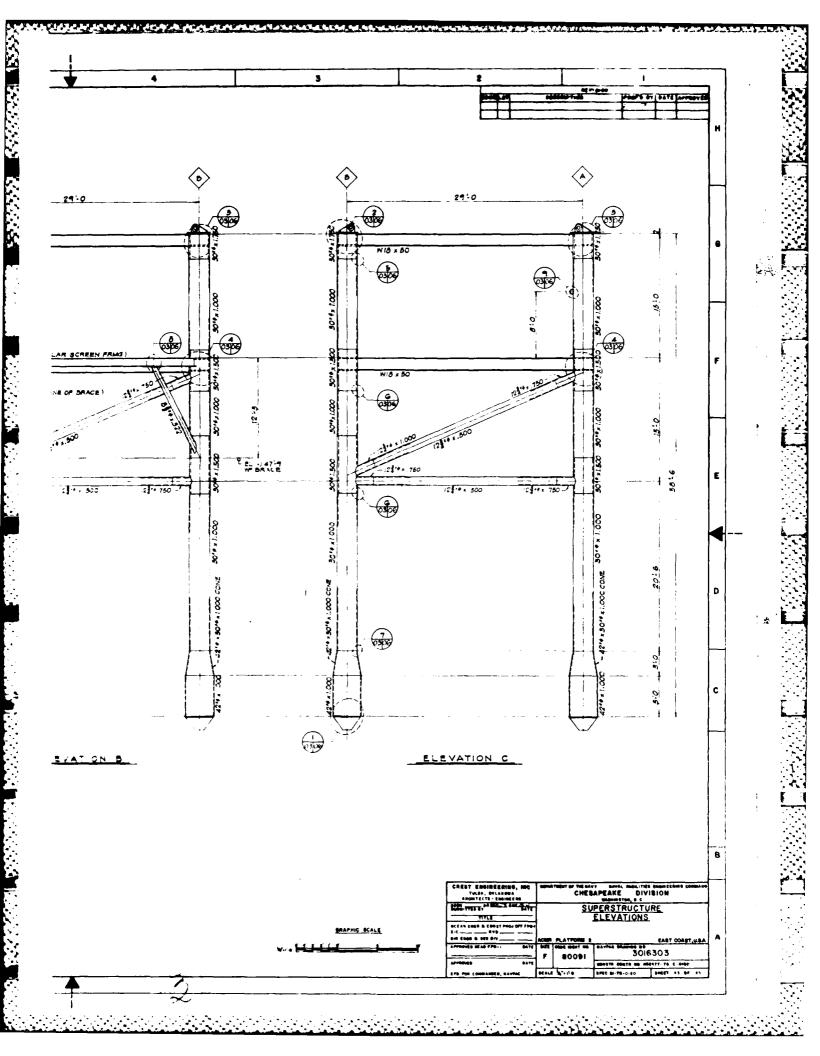


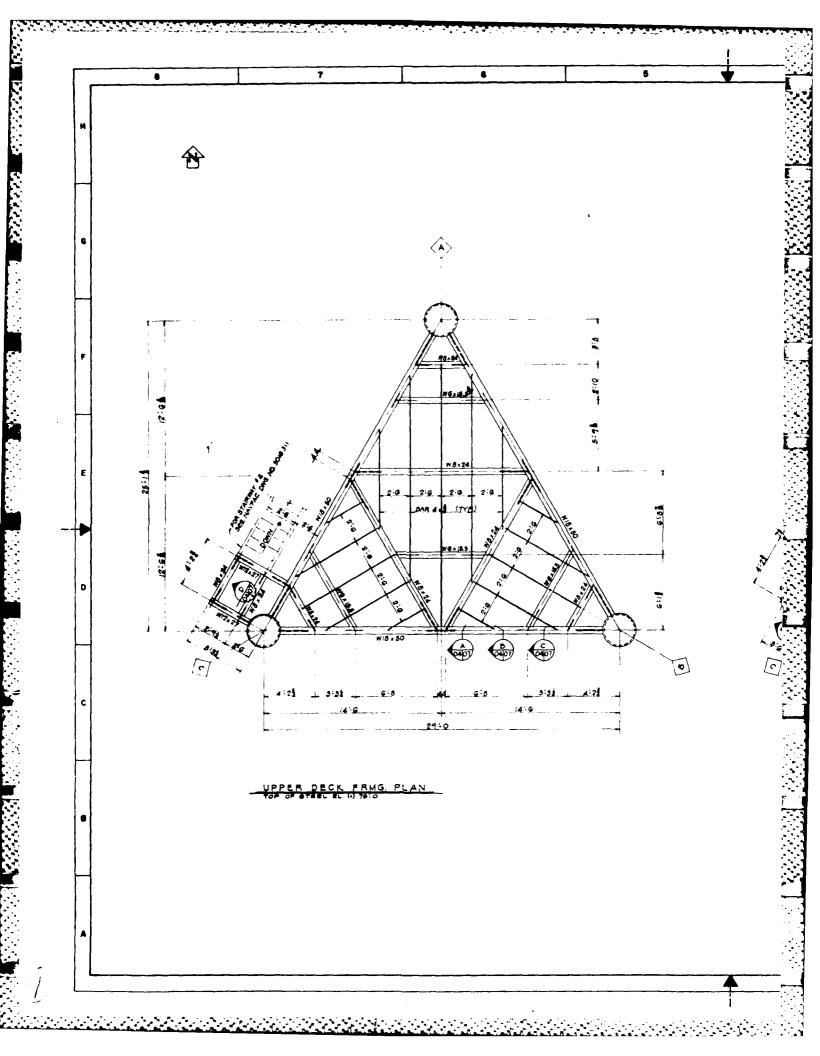


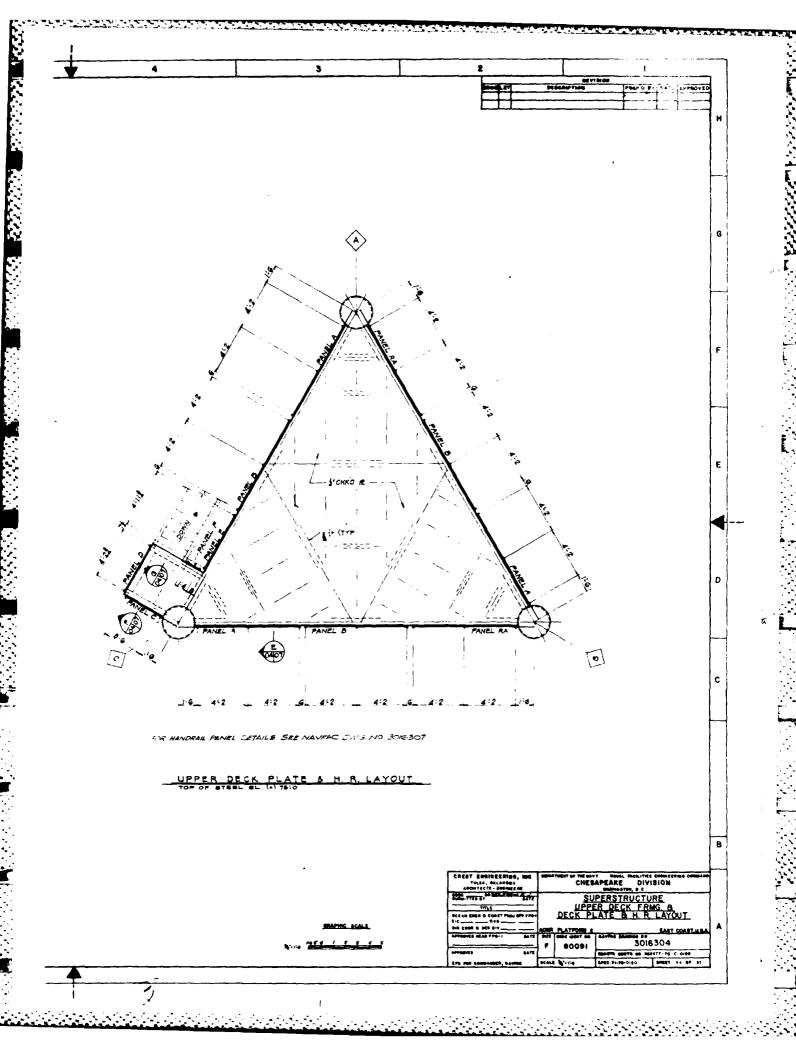


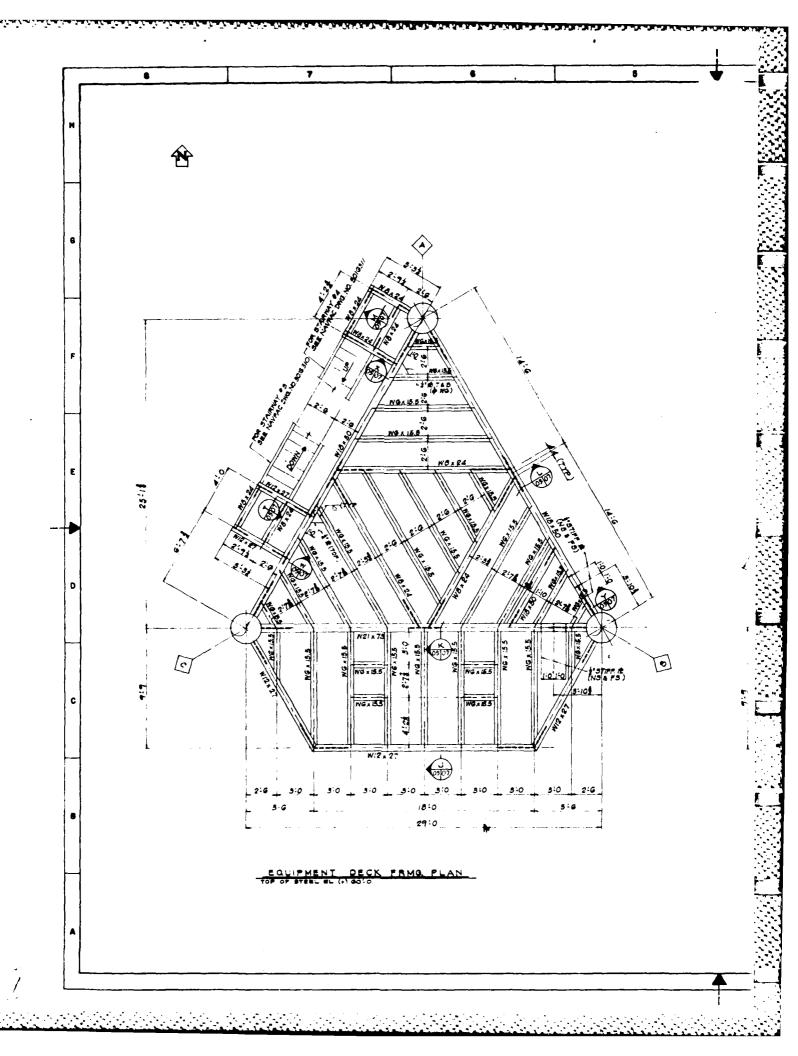




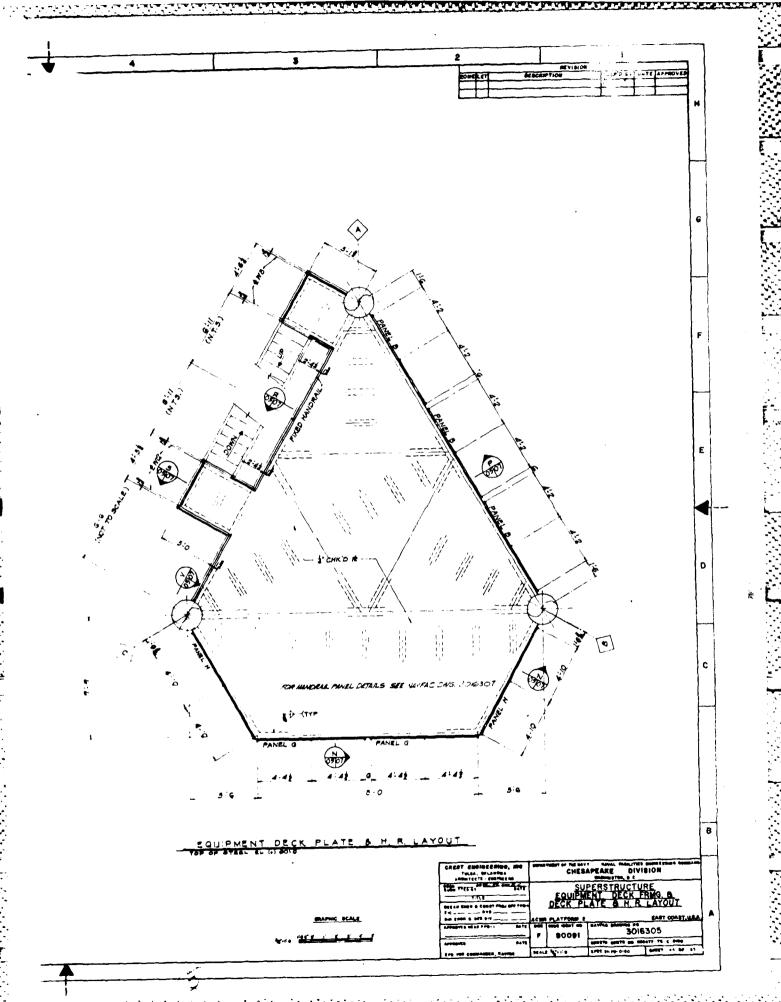








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SECTION 3.0
STRUCTURAL IDEALIZATION

3.1 INTRODUCTION

This section presents the mathematical structural model used for the analysis of the 93 feet MLW structure.

The structure is modeled as a space frame. Joint coordinates and member incidences are generated, as illustrated in Section 3.2, to obtain an efficient computer model. The model is then used in the SEALOAD program to generate the wave loads applied to the structure during the 50 year storm. Finally, the model is used in the STRAN program with the wave loadings produced by SEALOAD to analyze the structure for the 50 year storm.

To fully represent the jacket's structural behavior, dummy members are used to simulate the pile-jacket interaction. These members are modeled so that only shears perpendicular to the piling are transferred between the jacket and the piling.

The pile-soil interaction is considered in STRAN through the Coupled Interaction Analysis feature. This achieves convergence between the boundary conditions of the nonlinear pile foundation and the linear structure. The input data required for this feature is found in Section 3.5.

In STRAN the individual structural members of the mathematical model of the structure are not given distinct integers for identification. Each structural member is identified by the joint number at the beginning of the member and the joint number at the end of the

member. Therefore, Member 701-703 is that member of the model connecting Joint 701 to Joint 703. The member start is Joint 701 and the member end is Joint 703, and therefore, the local (member) x - axis is positive toward Joint 703.

Also in STRAN, member properties are designated through GROUPS. Each GROUP has a unique set of member properties, and each member of the model is assigned to a particular GROUP with the member incidence card. A list of the GROUP designations is found in Section 3.3. The member properties of each GROUP are listed in Section 3.7.

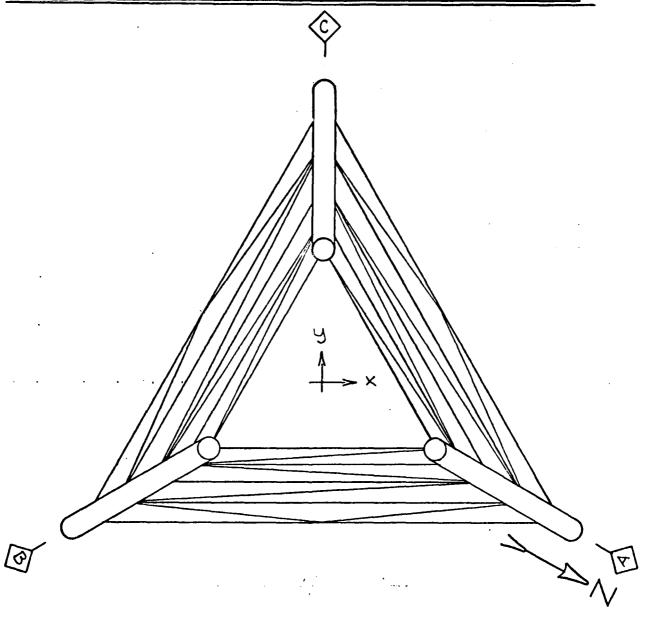
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3016291	Jacket - Plan at El. (+) 12'-0"
3016292	Jacket - Plan at El. (-) 13'-0" & (-) 39'-0"
3016293	Jacket - Plan at El. (-) 66'-0" & (-)93'-0"
3016303	Superstructure - Elevation
3016304	Superstructure - Upper Deck Framing
3016305	Superstructure - Equipment Deck Framing

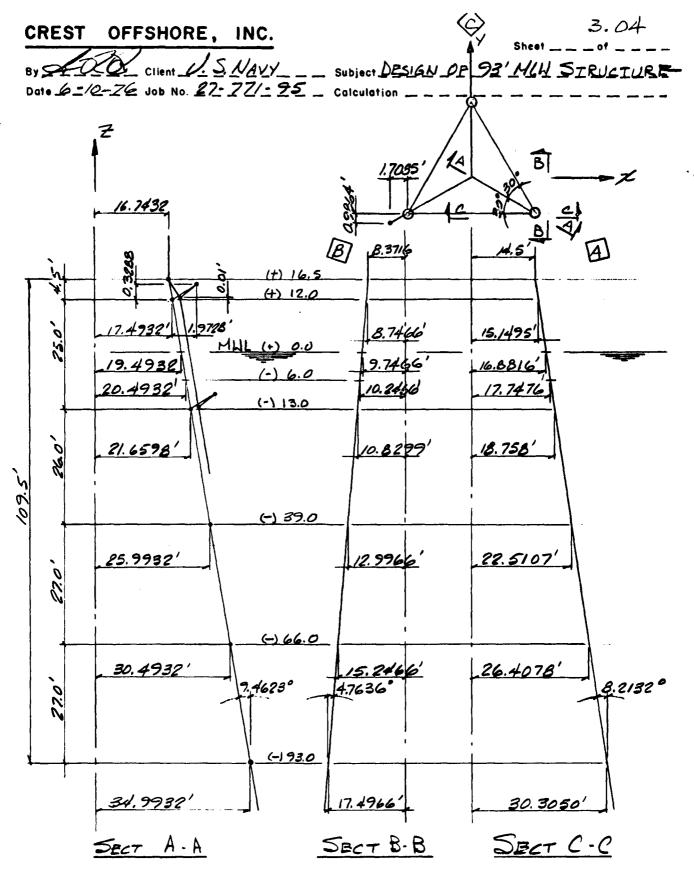
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By L. KIRK Client U.S. MANY _ Subject DESIGN OF 93' MLW STRUCTURE Date 6-30-76 Job No. 27-771-95 _ Calculation _ _ _ _

3.2 SKETCHES - PLANS AND ELEVATIONS

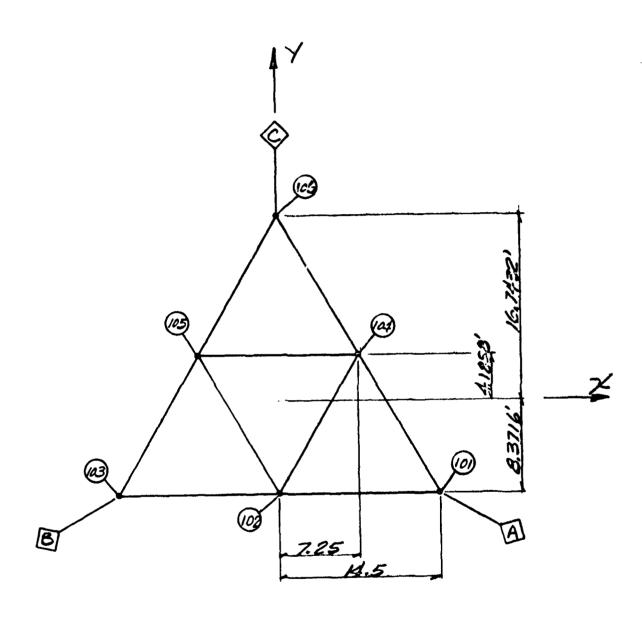


KEY PLAN

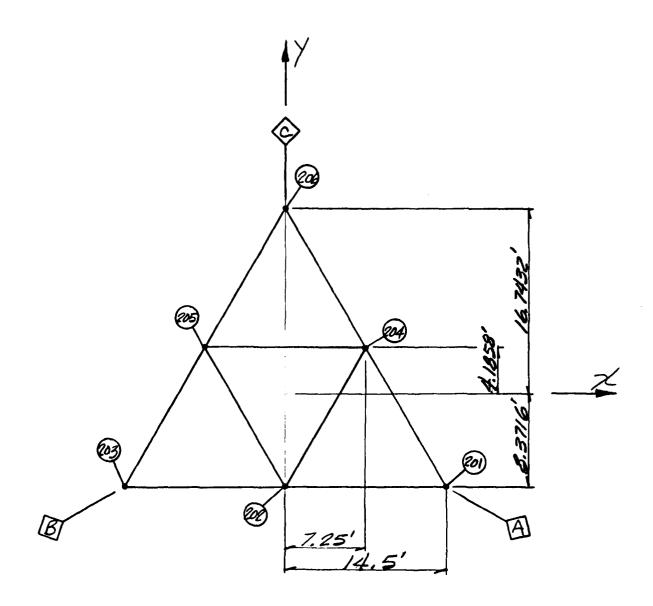


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Sheet ____of ____ By A CON Client U.S. NAYY __ Subject LESIGN DE 93' MLL STRUCTURE Date 6-29-26 Job No 22-221-95 _ Calculation ______



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Date 6-22-26 Job No. 22-721-25 _ Calculation ______

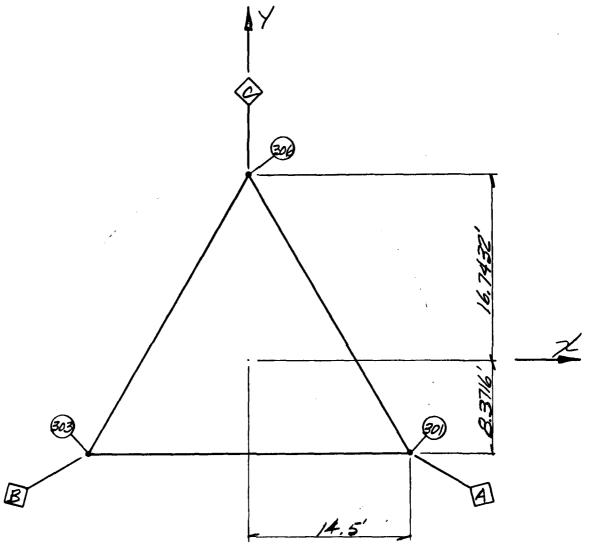


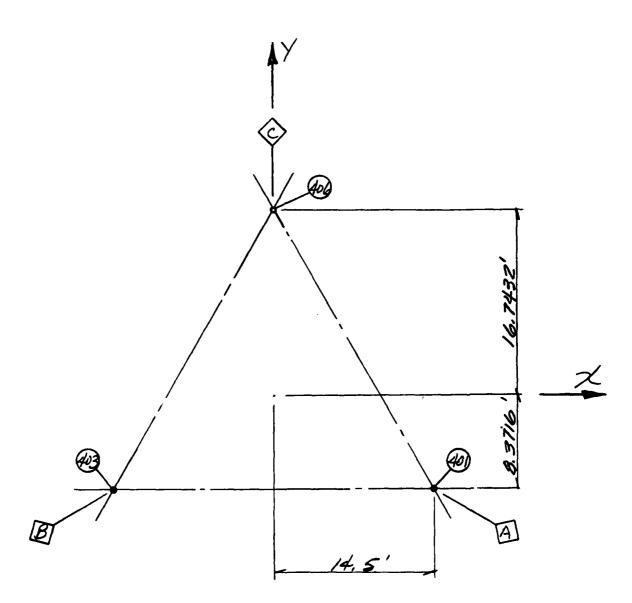
CREST OFFSHORE, INC.

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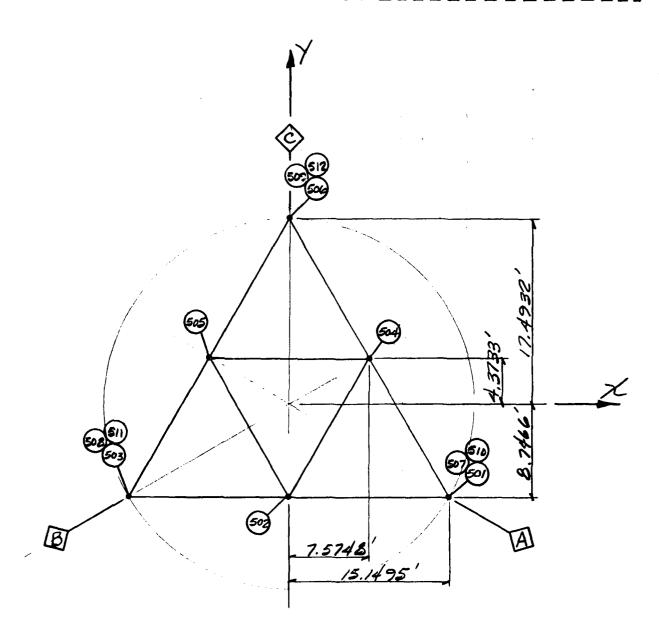
Client U.S. MAYY ___ Subject _ DESIGN OF 93'MW STRUCTURE

Date 6 = 22-26 Job No. 22-721-95 _ Calculation ______



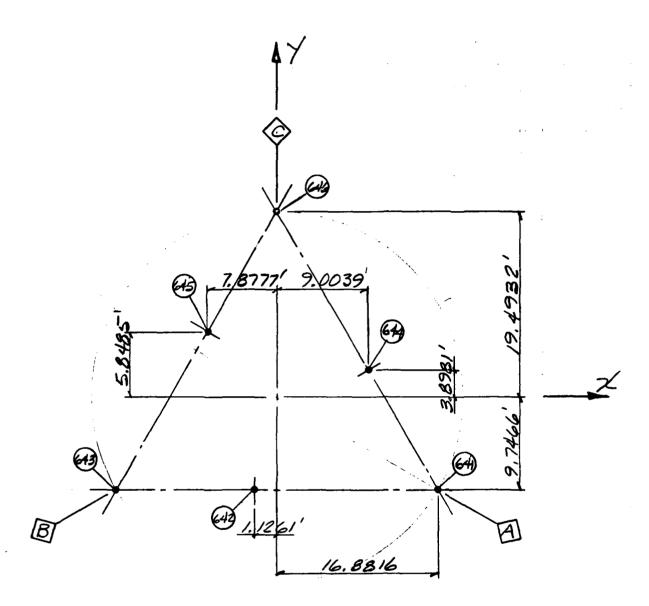


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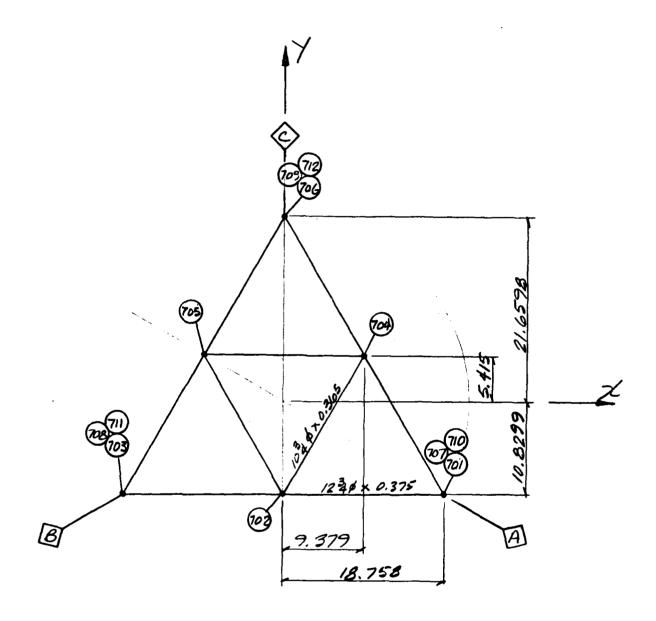
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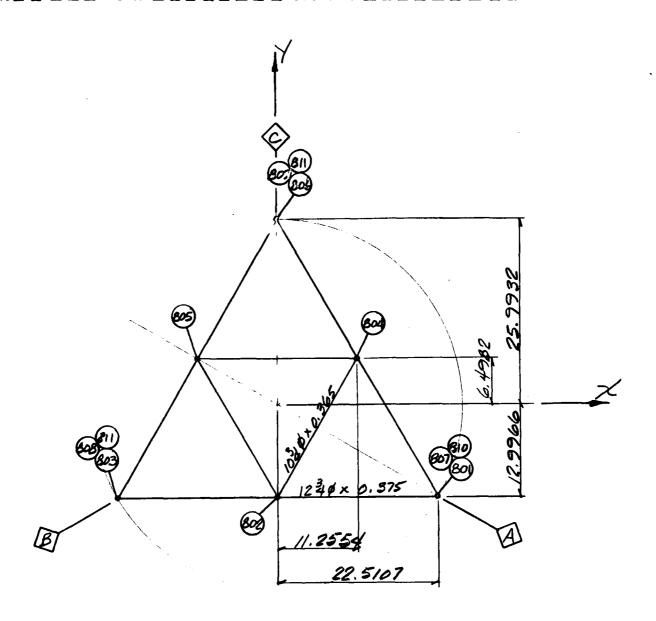


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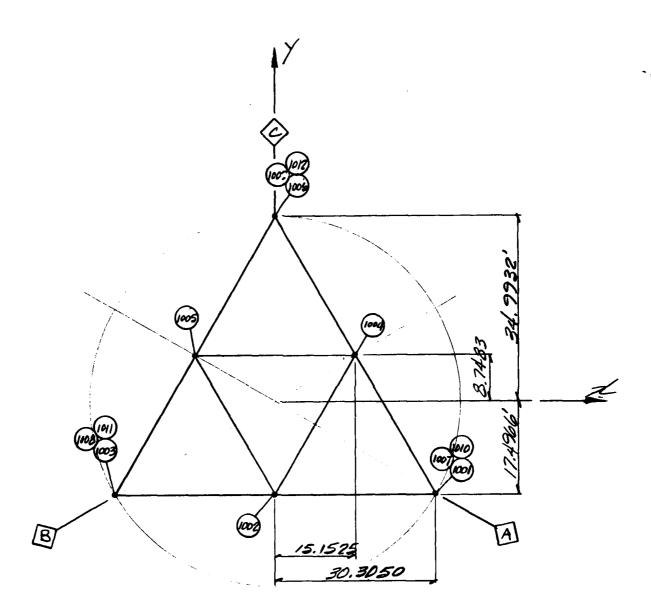
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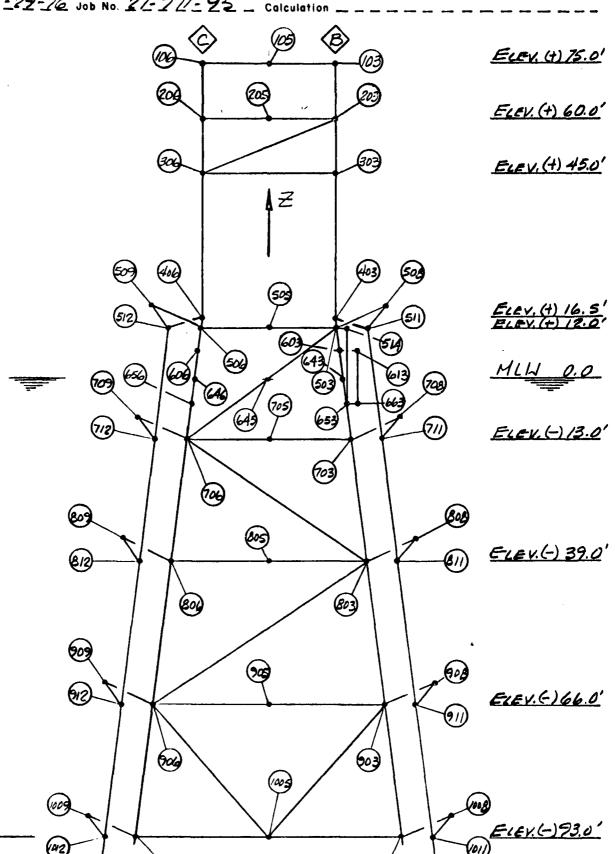
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By STOCK. Client U.S. NAVY __ Subject DESIGN OF 93' MLW STRUCTURE Date 6-22:26 Job No. 22-22/-95 _ Calculation _____



CREST OFFSHORE, INC.

By FICH L.S. MANY _ Subject DESIGN DE 93' MILL STRUCTURE
Date 6-29-76 Joh No. 27-771-95 Colculation



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SECTION 4.0 BASIC LOADS

4.1 INTRODUCTION

This section presents the loads which are applied to the structure.

Section 4.2 contains the estimated weight of the structural material not considered by SEALOAD because of the structural idealization of the model.

Section 4.3 contains the calculations for live loads applied to the Upper Deck and to the Equipment Deck.

Section 4.4 illustrates the data required for the wind loads feature of SEALOAD. The wind loading applied to the individual structural members of the model is found in Appendix B.2.

Section 4.5 contains a summary of the shear force and overturning moment at the mudline for each wave direction. The wave loading applied to the individual structural members of the model is found in Appendix B.2

4.02

By A DO Client U.S. NAVY _ Subject DESIGN DE 23' MLH STRUKTURE
Date 7= 9-76 Job No. 27=771=25 Calculation _BASIC LOADS _____

4.01 DEAD LOADS

- 1. Height of the structure is considered in the SEALOAD 2 analysis.
- 2. Top Deck! A 15 kip load was assumed as the weight of the deck. The load was distributed uniformly at each column
- 3. Equipment Deck: A 15 kip load was assumed as the weight of the deck. The load was distributed uniformly at each column.
- 4. Boat Landing: // kip load (estimated)
 was applied at each support.
 A total of 22 kips was applied.
- 5. Boat Bumper: 1.2 kip load (estimated) was applied at each Bumper.

4.2 LIVE LOADS

1. Top Deck: 100 psf load was applied to a 364 FT area and distributed uniformly along the 5 WIBESO.

100 × 364 = 418.4#/F

By A DO, Client U.S. NAVY Subject DESIGN DE 93' MLL STRUCTURE
Date 7=9-74 Job No. 21 = 271-95 Calculation BASIC LOADS

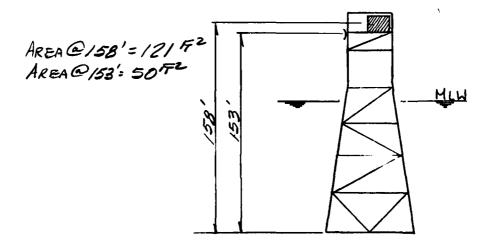
4.2. LIVE LOADS (CONT'S.)

2. Equipment Deck: 150 psf load was applied to a 364 Ff area and distributed uniformly along the 3 main support beams.

a) Equipment Deck Cantilever: 150 psf load was applied to a 192 FT area and distributed uniformly along the WZI@73 support beam.

4.3 Wind Load

- 1. Wind on the structure is considered in the SEALOAD 2 analysis.
- 2. Wind as applied to the solar panel and antenna



4.04

By SECON Client U.S. NAVY __ Subject DESIGN OF 23' MUN STRUCTURED Date 7 = 9 - 76 Job No. 27 = 271 - 25 Calculation _____

4.4 WAVE LOADS

The wave loads on the members of the platform are calculated by the SEALOAD-2 program using Dean's Stream Function wave grid profiles.

A summary of the shear force and overturning moment at the mudline for each wave direction selected follows on the next four pages. Note that these forces and moments also include the wind loads.

Sheet of

By U. Talkat Client U.S. Navy _ subject Design of _ MLW Structure
Date 9-3-76 Job No. 27-77/- _ Calculation _ Wave Loads _ _ _

The roughness effect of the morine fouling for that part of the structure from the Mean Low Water to the Mudline is considered by increasing the effective diameter used in SEALOAD to increase the drag. However, this results in a larger inertial force being applied to the structure. Therefore, the mass coefficient is reduced correspondingly. The following equations are used to determine Deff and Cm used in SEALOAD.

Dact = D + 2"

Deff = Dact × 1.02/.74

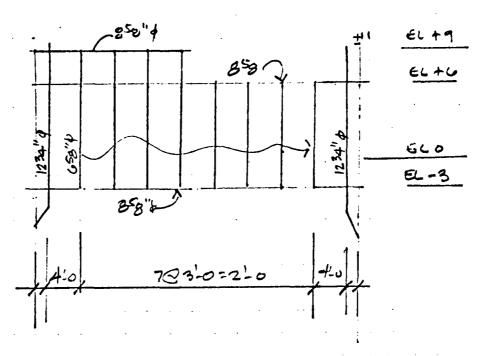
Cmeff =
$$\frac{Dact}{36 \times Deff}$$

These equations produce the following table.

D	Dact	De+F	<u>Cm</u>
103/4"	12.75	17.57	0.257
1234"	14.75	20.33	0.297
14"	16.00	22.05	0.322
16"	18.00	24.81	0.363
18"	20.00	27.57	0.403
20 "	22.00	30.32	0.443
45 12"	47.50	65.47	0.957
472"	49.50	<i>68.</i> 23	0.997

4.06

BOST LANDING WAVE AFEA



SUPPLIE APEN FRONT FACE

HOPIZONTILD $1.6569 = 0.12 + 10 = 11.52 + 10.52 + 10.52 = 0.72 \times 29.12 = 41.70 = 53.28$ VEFT. $2.124 = 1.0625 \times 2.715 = 38.34 + 65 = 0.552 \times 4 \times 13 = 28.70 + 10.51 = 22.03 = 29.12$

3ACK FACE

HOR, 1-68'\$ = 0.552 × 16' × 1 = 8.83 SIF,

2-88'\$ = 0.720 × 29 × 2 = 41.70 "

VER 3-68\$ = 0.552 × 9 × 2 = 9.94 "

2-68\$ = 0.552 × 13 × 2 = 14.35 "

ASSUMING BACK FACE IS SHIELDED SOMEWHAT

TETAL AFEA = 142,4+ 0,5 × 74.88 = 179.84 5,5,

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SECTION 5.0

LOADING CONDITIONS

5.1 INTRODUCTION

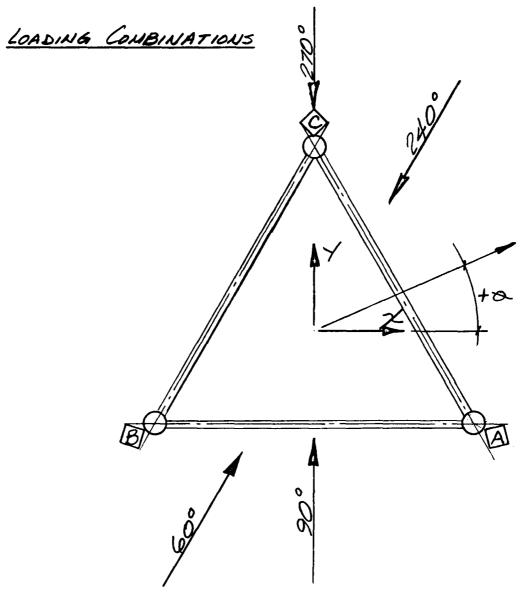
This section describes the wave approaches considered and the loading conditions used to analyze the structure for the 50 year storm.

Loading Conditions 1 to 4 are the load conditions generated by SEALOAD for the maximum force on the structure (in the area of the wave crest) for the four selected wave approached. Load Condition 5 is the dead weight generated by SEALOAD, the dead weight not included by the model, and the live load on the two deck areas. Load Conditions 6 to 9 are the maximum wave load conditions, Load Conditions 1 to 4, added to Load Condition 5, the total dead weight and live load of the structure.

Sheet __ _ of __ _ _

By A DB Client U. S. NAVY Subject DESIGN OF 93' MINI STRUTUSE

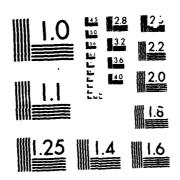
Date 7-9-76 Job No. 27-721-95 Calculation _____



LOAD CASE 1: Wind and Wave at 60° LOAD CASE 2: Wind and Wave at 90° LOAD CASE 3: Wind and Wave at 240° LOAD CASE 4: Wind and Wave at 270° COAD CASE 5: DEAD LOADS + LIVE LOADS

LOAD CASE 6: LOADING 1 + LOADING 5 LOAD CASE 7: LOADING 2 + LOADING 5 LOAD CASE 8: LOADING 3 + LOADING 5 LOAD CASE 9: LOADING 4 + LOADING 5 SECTION 6.0
SPACE FRAME ANALYSIS

DESIGN CALCULATIONS 93' MLW STRUCTURE EAST COAST AIR COMBAT MANEUVERING R. (U) CREST ENGINEERING INC TULSA OK SEP 76 27-771-95 CHES/NAVFAC-FPO-7614 F/G 13/13 ND-A165 689 217 NĽ UNCLASSIFIED



MICROCOPY RESOLUTION TEST CHART

6.1 INTRODUCTION

This section contains the results of the space frame analysis of the structure subjected to the specified environmental conditions.

The space frame analysis set forth herein utilizes the available computer programs available at Synercom Technology, Inc., Houston,

Texas. The program processing procedures are as follows:

- Set up SEALOAD-2 program to obtain desired wind, wave and dead weight (including buoyancy effect) loadings on the structural components.
- Update loadings in Step (1) due to additional dead weight and live loads on the structure.
- 3. Perform space frame analysis by using STRAN computer program.

By A Client U.S. NAYY Subject DESIGN OF 93' MILL STRUCTURE
Date 9-3-76 Job No. 27-771-95 Calculation SPACE FRAME ANALYSIS

6.2 MAZIMUM MEMBER STRESSES

The following three pages tabulate the maximum stress each member of the mathematical model experiences. The loading condition in which this stress occurs as well as the maximum unity check are indicated. Since one-third increase of all allowable stresses was used by the computer due to the storm condition, all unity checks should be compared to 1.00.

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6.3 MAXIMUM MEMBER FORCES

The following three pages tabulate the maximum member forces for each member of the mathematical model. Since a one-third increase of all allowable stresses was used by the computer due to the storm condition, all unit, checks should be compared to 1.00.

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TAN TENDER STRESS REPORT NO.

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CREST OFFSHORE, INC.

Sheet ____ of ____

By ADES Client U. S. NATY _ Subject DESIGN OF 93' MIN STRIKTURE
Date 2-3-76 Job No. 27-771-25 Calculation SERGE FRAME FINALYSIS_

6.4 JOINT DEFLECTIONS AND ROTATIONS

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CREST OFFSHORE, INC.

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6.5 REACTIONS

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SECTION 7.0
TUBULAR JOINT ANALYSIS

7.1 INTRODUCTION

This section contains the analysis of all of the tubular joints of the structure. The tubular joints are separated into two groups, the Primary Joints and the Secondary Joints.

The Primary Joints are those joints involving the jacket legs. Section 7.2 displays the joint geometry and location for each of the Primary Joints. This section serves as a key to Section 7.3, the computer analysis of the Primary Joints.

The Secondary Joints are those joints involving the interior bracing at each of the horizontal levels. Section 7.4 displays the joint geometry and location for each of the Secondary Joints. This section serves as a key to Section 7.5, the computer analysis of the Secondary Joints.

The computer program used for the tubular joint analysis of this structure is a post-processor program for STRAN developed by Crest Offshore, Inc. This program is based on AISC and API criteria for stress in tubular members.

Reference Drawings:

3016290 Jacket - Elevations

3016291 Jacket - Plan at El. (+) 12'-0"

3016292 Jacket - Plan at El. (-) 13'-0" & (-) 39'-0"

3016293 Jacket - Plan at El. (-) 66'-0" & (-) 93'-0"

3016303 Superstructure - Elevations

OFFSHORE, INC. CREST By L. KIRK __ Client U.S. MANY _ _ Subject DESIGN OF 93' MLW STRUCTURE
Date 1-29-76 Job No. 27-771-95 _ _ Calculation TUBULAR JOINT ANALYSIS __ (+-) 75-0 JOINT GEOMETRY-PRIMARY JOINTS (+) 60-0" 62.70 -<u>(+) 45[']-0</u>" TYPICAL ELEVATION 82.00° <u>(+)</u> 12-0" 45**.59**° 62.218 <u>(-) 13-0</u> 50**.3**0° 64.52 (<u>-</u>) 39⁻0" 69.02° 52.40° (-)66-0 51.58 51.58 -<u>(-) 9 3</u>-0''

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CREST OFFSHORE, INC.

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By SELLON_ Client U.S. NAVY _ Subject DESIGN OF 93 MILL STRUCTURE.

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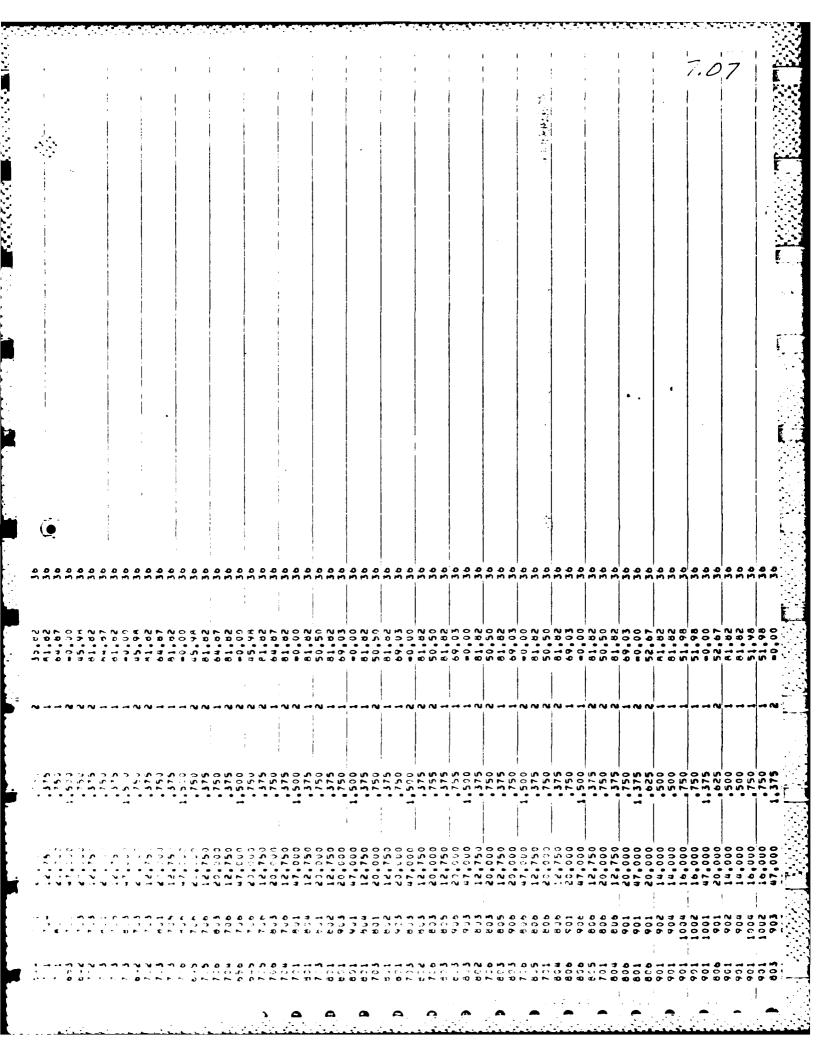
7.3 PUNCHING SHEAR ANALYSIS - PRIMARY JOINTS

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AXIAL	4,349	1,108	3,709	3,056	3,00 m	1,510	3,750	77	5.731 1.108 5.047	M 704 W 709 W 709 W 116	3 4 7 2 0 5 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8
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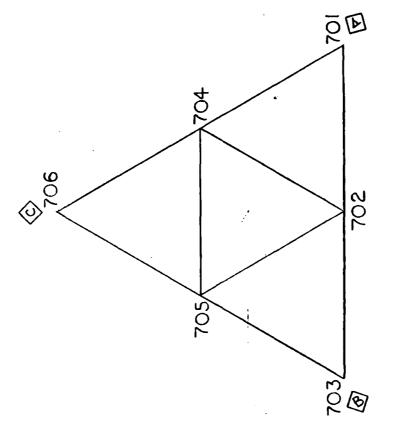
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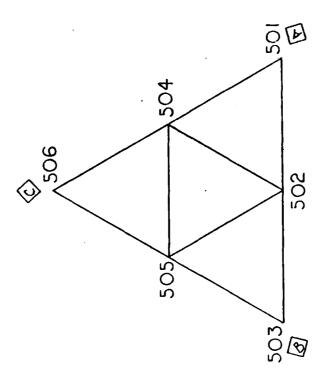
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PUNCHING	A H A H		40.154	3,279	1,689	5,759	3,637		•	•		5,434			2.992	3,178		3,948	4,773		~	2,627		3,541	•		6,027	6,539		3.957	876 7		5,605	6,107
BENDING		109.	560	007.2	1.603	2,369	5,945	1,000	5.224	3.296	1000	2,650	55	175	690.9	6,339	155	2,405	4.097	.157	5,314	067	,327	1.599	3.620	667	3.400	690.4	.326	1.948	3.801	20 27 4	2,774	3,790
AXIAL		474	12,351	966.9	3,243	13,318	6,883	77	11.068	6.926	7,056	11,919	11,828	0.014	219	358	467	5.925	5,875	.001	.067	£ 70°		~	•	545	9,300	9,509	007	962.9	6.530	.564	8,904	6.947
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THICKNESS /* *8 T	1,250 ,369 ,625 5,746 ,625 5,858	1,250 .563 .625 .9.275 .625 .6.736	1,250	1,250 ,564 ,625 9,257 ,025 8,988	1,250 .642 .625 10,523 .625 10,185
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By L. KIRK _ Client U.S. NAVY _ Subject DESIGN OF 93' MLW STRUCTURE Date] -28-76 John No. 27-771-95 _ Calculation TUBULAR JOINT ANALYSIS _





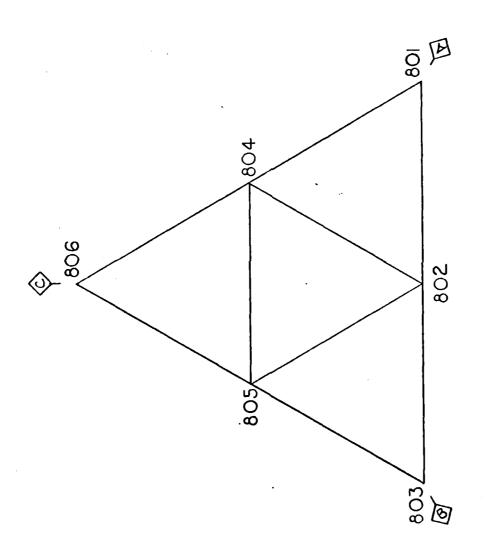
JOINT GEOMETRY - SECONDARY JOINTS

AT ELEV. (+) 12'-0"

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By L. KIRK _ Client U.S. MAYY _ Subject DESIGN OF 93' MLW STRUCTURE Date 7-28-76 _ Job No. 27-771-95 _ Calculation TUBULAR JOINT ANALYSIS _



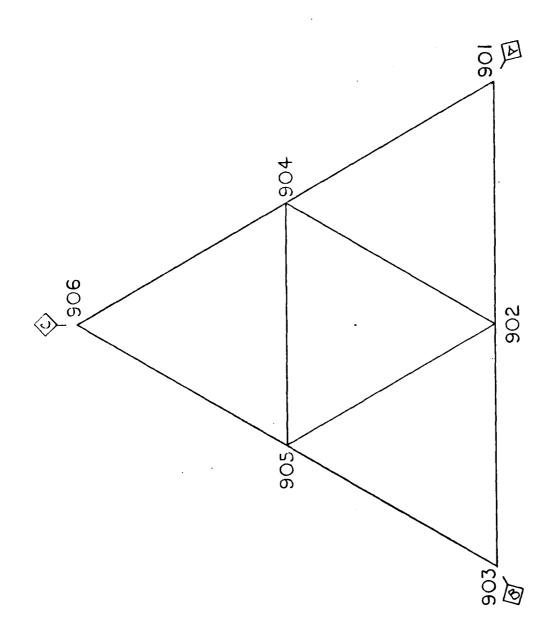
JOINT GEOMETRY — SECONDARY JOINTS

ELEV. (-) 39'-O"

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By L. KIRK _ Client U.S. MAYY _ Subject DESIGN OF 931 MLW STRUCTURED Date 7-28-76 John No. 27-771-95 _ Calculation TUBULAR JOINT ANALYSIS

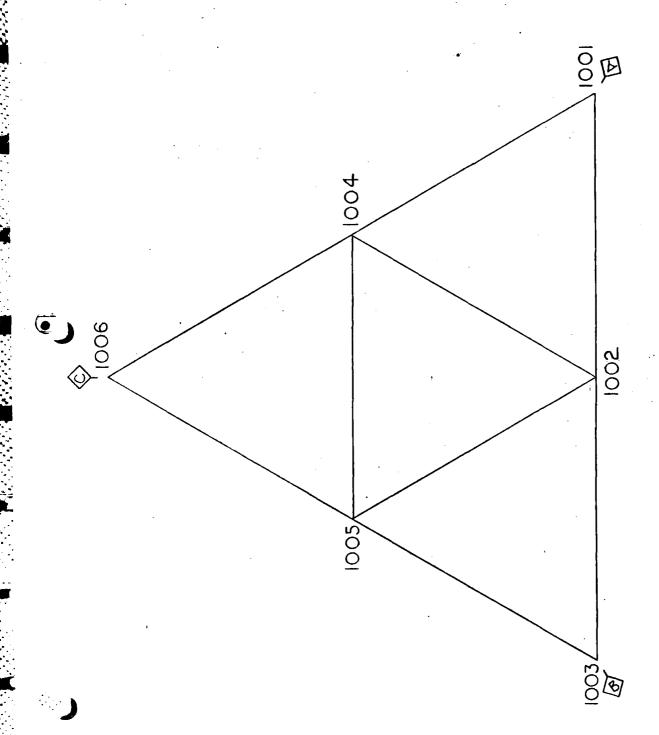


JOINT GEOMETRY - SECONDARY JOINTS

ELEV. (-)66'-O"

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By L. KIRK _ Client U.S. MANY _ Subject DESIGN OF 93' MLW STRUCTURE Dute 7-28-76 John No. 27-771-95 _ Calculation TUBULAR JOINT ANALYSIS _

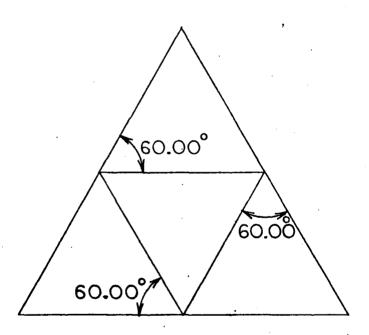


JOINT GEOMETRY - SECONDARY JOINTS

PLAN AT ELEV. (-) 93 '-O"

Sheet	 _	_	of	_	 	

By L. KIRK CHEM U.S. MAYY Subject DESIGN OF 93 MLW STRUCTURE
Date 7-29-76 Job No. 27-771-95 Calculation Tubular Joint Analysis



TYPICAL PLAN

JOINT GEOMETRY - SECONDARY JOINTS

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SAPCHK - CREST OFFSHURE, INC. STRUCTURAL PUSTPRICESSUR SYSTEM

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SAPCHK - CHEST DFFSHURE, INC. STRUCTURAL PUSIPHOCESSUR SYSTEM

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SAPCHA - CHEST OFFSHURE, INC. STRUCTUMAL PUSTPAUCESSUR SYSTEM

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ESS + + C	4.510 2.123 4.731	2, 483 3,154 5,472	1.205 2.170 5.87¢	2.17d 5.444 5.605	2,292 5,170	2,740	2.43. 2.43. 3.054	400° a	2.424 5.185	,158 2,271 3,620	2.554 2.554 4.43
AXIAL	6.430 4665	7.600	7.246 .603	6.945 968 910	6.924 591 1.082	6.707	3.546	040	3.457	5.602	3.708
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AAIAL	5.328 6.651 .758	3.470	13.553	3.90A 12.216 332	5.354 13.258	3a5v6 11.846	5,229	5.965 12.264	5,379 13,327	6.046 11,928	7.150
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SAPCHA - CREST OFFSHUME, INC. STRUCTUMAL PUSTPROCESSUR SYSIEM

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	• •					SHEAR	SHEAK
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1001 1004 1004	901 1004 1002 1004		. 475 . 750 . 375	026 6.879 017	2 4 5 3 4 5 4 5 4 5 4 5 4 5 4 5 4 5 4 5 4	1.081	6,010
1971 1004 1064	1002 1004	24 00 10 00 14 00	875 750 375	3,270 11,455	2,78 2,756 3,452	7,420	0.816 8.733
1004 1006 1004	400 1504 1004 1505	24.00 10.00 14.00	, 875 , 750 , 575	5,551 7,153 ,741	2.700	5.029 1.138	8.010 0.733
1004 1006 1004	7 906 1004 1004 1005	24.00 10.00 14.00	. 875 . 750 . 375	6.178 11.984	2,107	7.132	8,816 6,733
1004 1006 1004	906 1908	10.00	875 750	5,540 6,842 723	2.574 2.574	1,163	8.816 8.733
1004 1006 1004	906 1004 1004 1005	24.00 16.00 14.00	. 475 . 754	6,24A 11,836	1.642	6.916	8,616 8,735
DIEND OF JUINT CHECK							

SECTION 8.0
PILE-JACKET CONNECTION

8.1 INTRODUCTION

This section ascertains the capability of the pile-jacket connection at the top of the jacket of transferring both the axial load and the bending moment of the jacket to the pile. The following assumptions are made in this analysis:

- 1. The axial load is distributed to all six shims.
- The resultant bending moment is taken out as a couple by two shims on each side of the 42 inch diameter pile.
- 3. The torsional moment is negligible.
- 4. The fillet weld area perpendicular to the applied load is more susceptible to fatigue crack than the fillet weld parallel to the load, and, therefore, is ignored in this analysis.
- 5. An E-70 electrode is used for welding with τ = 15.8 ksi or f = 11.2 ω kips/inch.

Reference Drawings:

3016295 Jacket - Pile Shims and Leg Connection

By L.KIRK Client U.S. MAVY Subject DESIGN OF 93 MLW STRUCTURE
Date 8-2-76 Job No. 27-771- Calculation PILE- JACKET CONDECTION

STORM 50 yR.

20401

(IuKips) (IuKips) (IuKips) (Kips) (Kips) (Kips)	269.63 373.35 277.86 405.53	431.44 400.43 465.0 412.48	410.35 2 425.27 3 94.86
P/6 (K1PS)	262.64 66.97 217.09 5176.26 248.87 30.55 203.37 202.17	13.3.55 \$272.91 216.72 \$183.63 151.72 \$14.22 214.15 193.33	76.06 33467 04.71 520.64 k 77.62 6317.86 33.77 6 34.956
MR 42".2 (KIPS)	262.66 217.09 248.87 203.37	133.55 27.22 151.72 21.4.15	76.06 24.71 77.62 33.77
(In Kips)	2015,24 21969,91 22063,11 3517,46 17892,93 (8235,44 -2373,35-20769,4 20905.39 -3760.33-166,336 17032,97	2135.41 -1/438.53 1/535.21 3151.52 -17933.43 13923.21 -27456 12445.15 12744.45 -5444.03 17456.05 17983.65	6339.21 7555.03 6519.73 7037.03
MZ (I.v KiPS)	2015, 24 21969,91 22063,11 3517, 46 17892,93 (8235, 44 -2373,35 -20769,4 20905,39 -3760,33 -166,396 17032,97	2135.41 -11436.53 11535.21 3151.52 -17933.43 13.003.31 -27456 12445.15 12.744.43 -3444.03 17156.05 17983.65	2005.73 -6189.6 -1583.77 6339.21 28.43.31, -757.43 2355,43 7556.03 -1503.57 5501.73 2770.75 6519.73 -2213.74 6705.93 -2130.01 7037.03
(IuKips)			-6/89.6 -7:57.4/3 550/.73 (5705.93
(KIPS)	-4.83 -1057.55 185.93 1212.99	-/757.43 -//01.96 /835.67 //39.95	2005.73 22,13,36 -1503,57 -2213.74
LOAD	2100	9 2 8 2	2000
VACKET	105-104	403-503	703-507

20 0000 TOTAL LOAD IS MAXIMON SHIM NOTE:

CREST OFFSHORE, INC.

2.05

By ACO Client V.S. MAYY Subject DES'GH DE 23' ML STRUCTURED Date 8:5-26 Job No. 27:271-25 Calculation ENE JACKET CONNECTION

1/10TH OF SHIM

(48-3.5) TT -3.5 = 19.8

USE: 6~1"PX 19.75" WIDE

8.3 CHECK JACKET TO SHIM

EFFECTIVE WELD = 1.75"-0.0625" = 1.6875

ALLONABLE (0AD = 15.8 (0.70711) (1.6875) = 18.85

SLOT LENGTH = $\frac{485.27}{16.85(2)}$ = 12.87

8.4 CHECK SHIM TO PILE

SHIM PLATE = 1.0" THICK

MAXIMUM WELD = 1.0 - 0.0625 = 0.9375

ALLOWABLE LOAD = 15.8 (0.70711) (0.9375) = 10.47

LENGTH OF SHIM READ, = 485.27 = 23.16

8.5 CHECK SHIM STRESS

SHIM AREA = 19.75"2 D = $\frac{485.87}{19.75}$ = 24.56 < 36x.6x1.53 = 23.7 dc

8.6 CHECK JACKET STRESS

JACKET EFFECTIVE AREA = 19.75 x 1.6875 = 33.33 T. A = 485.27 = 14.55 < 28.7 d SECTION 9.0
PILE ANALYSIS

9.1 INTRODUCTION

This section determines the pile penetration and pile schedule of Structure 2.

First, the actual maximum pile loads are calculated. Then, these loads are used with the Pile Capacity Curves to establish the penetration required. Finally, the Pile Driving Resistance Curves are checked to insure that the piling can be driven to the desired penetration.

The pile schedules are devised to avoid any possible set-up problems while driving and to minimize field welding of the pile add-ons.

In addition, the P-Y curves for the site are included in this section. The P-Y curves are used in Section 6.0 for the space framing analysis.

The Pile Capacity Curves and the Pile Driving Resistance Curves are from the Foundation Analysis, Report No. 27-771.97.

Reference Drawings:

3016302 Jacket - Pile Details

9.08

By ADOL Client 1.5. MAY _ subject DESIGN OF 33' MIN SELECTION Date 3-23-230b No. 22-221-25 Calculation _ PULE AMILYSIS _ ____

9.2 PILE SUMMARY

SITE PR

Pile Axial Loads

Maximum Compressive load 2914 "
Maximum Tension Load 1985.07"

Piling Dimensions

Outside Diameter 42 in.

Penetration below Mudline (S.F.=1.5) 275 F

Penetration below Mudline (S.F.=1.35) 255 F

Minimum Wall Thickness 2.0 in

Conclusion

The pile schedule with the 2.0 in. uniform wall thickness must be used to assure drivability to required penetration of 275 F for a safety factor of 1.5. In addition, equipment must be available to remove the internal plug by a controlled jettion and driving procedure should the need arise. No insert piling should be required.

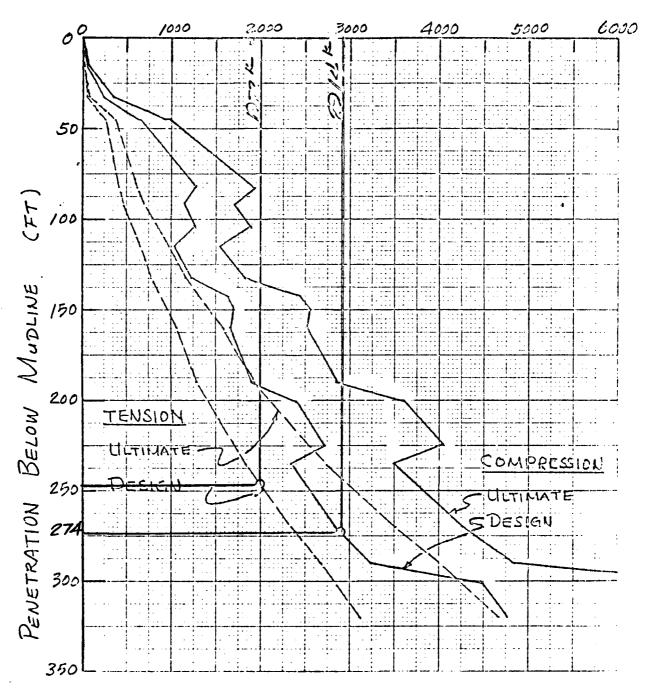
CREST OFFSHORE, INC.	
By Stor Client LLS. NAVY Subject DESIGN Date 2-3-26 Job No. 22-221-25 Calculation Puls	OF 73 MIN STRUCTURE
9.3 PILE GARS	
Maximum Computer Compressive (Ref. Reactions, Id. Co. 7, Seel.	re Force = 2658.99 Fon (25, p6.25)
Weight of piling below Mudline 42" of x 2.0 - 200'x 0.85 42" of x 2.875 - 25'x 1.002 Splice Points - 32' x 0.27	45/7 = 170.8 25/7 = 75.2 35/7 = 95 2555
Total Maximum Compressive Pilo	Good = 2914 F
Maximum Computer Tension For (Ref. Reactions, Id Care 9, Section	cc = 2184.34" 6.5, p 6.27)
Weight of Piling below Modline	
Live Load on Docks Upper Dock - 0.42 to x 295 Equipment Dock-(1.5 1/17)(14.517)+(0.63/4)	= 12 h- (14.517) = 31 43 h-

Total Maximum Tension Pile Load = 1972"

Sheet _____ of _____

By C. Chern Client U.S. NAUX ___ subject Foundation Analysis ____ Date 6-1-76_ Job No. 27=771=97_ Calculation Pipe Pile Capacity Consist

9.4 PILE CAPACITY CURVE CAPACITY (KIPS)



42-IN. DIAMETER PIPE PILES (Boring #2)

Sheet _____of ___

By C. C/1222 Client U.S. NAVY __ subject Foundation Annivers

Date 6-25-76 Job No. 27-771=97 _ calculation Pile Driving Resistance Con.

9.5 DRIVING RESISTANCE CURVES

MLW = 93"

250 FT Penetration

Vulcan 560 Hammer Wt. of Ram = 60,000 lbs Rated Energy = 300,000 ft-lbs Hammer Efficiency = 0.75 Wt. of Pile Cap = 42,000 lbs

Spring Constant = 6.2 x 10⁶ lbs/in.

Damping Factor, side & tip, J = 0.15

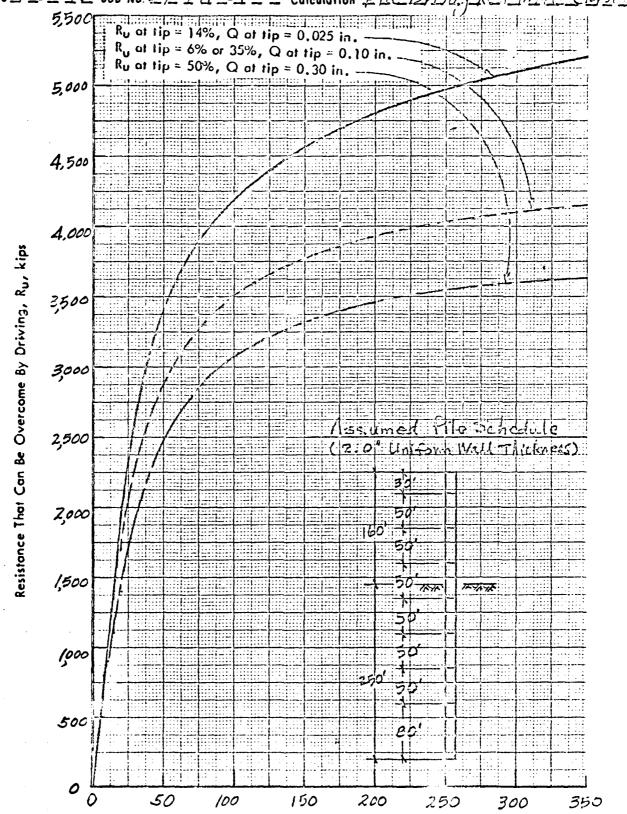
Quake Factor, side, Q = 0.10

Quake Foctor, tip, - See Above

	30-0"			2.0°m	P-3	
"0-,091	"0-'0E "C-'25"			2.0 "ur 2.0 um	P-7	
.091	0,05			2.0"WT	9-d	
	50.0"	20,02 30,00	-72	1 2 2 5 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	6-5	TF TATE
	1 "0-25		!	2.0"WT	p-4	
,,	"0-25 1 "0-,05 1 "0			2.0 "WT	P-3	
250,0"	,0-,05			2.0°VT	P-2	
	80,-0"			2.0 "WT	1-0	

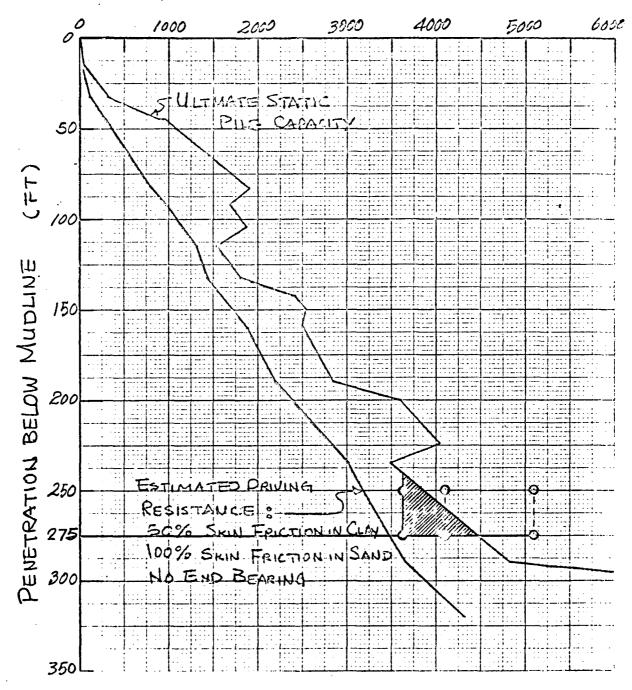
By C. Chern client 11.S. NAVY __ subject Foundation Analysic ______

Date 6-23-76 Job No. 27-271-01 _ calculation Pile Driving Residence Conve



Rate of Penetration, N, Blows per Foot 250-Ft Penetration

Sheen ____ of ____



2-in Uniform Wall

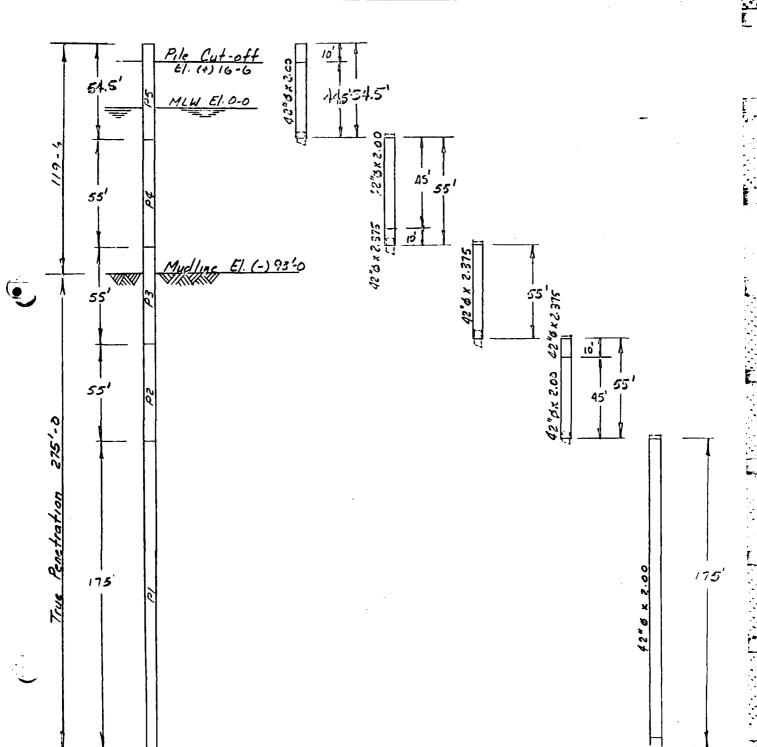
(Boring #2)
42-IN DIAMETER PIPE PILES

9.08 Sheet ___ of ___

By V. Tolbot Client U.S. Navy _ _ Subject Design of 93' Miles Structure _ _ Date 8-23-76 _ Job No. _ 27-771-95 _ Calculation _ Ble Analysis _ _ _ _ _

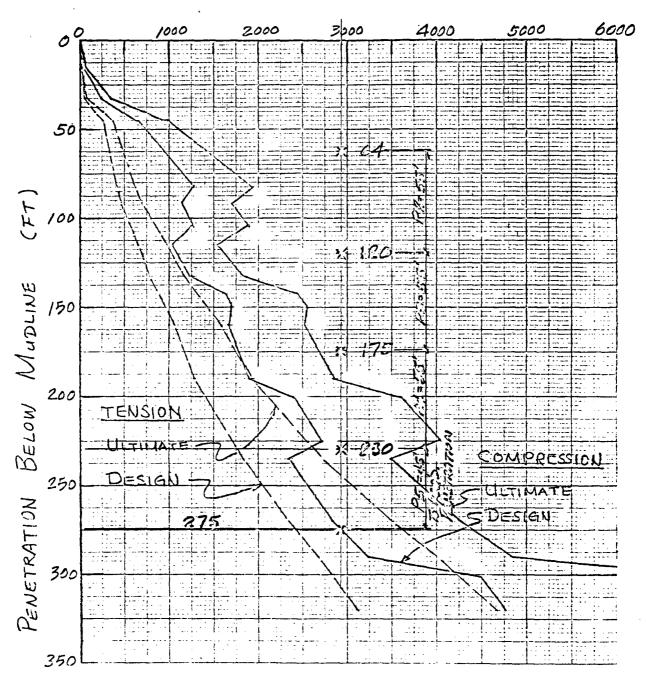
9.6 PILE SCHEDULE

Site #2



By C. Chern Client (1.S. NAUX ___ subject Foundation) Analysis _____ Date 6-1-76 Job No. 27-771-97 calculation Pipe Pila Capacity Curves ____

PILE CAPACITY (KIPS)



42-IN DIAMETER PIPE PILES
(Boring #2)

/, /L' Shcet _ _ _ of _ _

Top of belet

By V. Talbot _ Client U.S Mary _ _ Subject Design of 93' MCH Structure _ Date &-23-76 Job No. 27-771-96 _ Calculation _ Pile _ Analysis _ _ _ _

Check Maximum Length of Pile Add-on -

460k 454k

Weight of Hommer W/ Leads = 230 K

Using an impact factor of 2.0,

Total Vertical Load = \$60 K

Weight of Piling (42"x 2.375") = 0.084 %/in

Assume L = 57 ft = 684 in

 $f_{bhommer} = \frac{PL}{5} = \frac{77^k \times 684_{in}}{2400 \text{ in}^3} = 22.0 \text{ ks}$

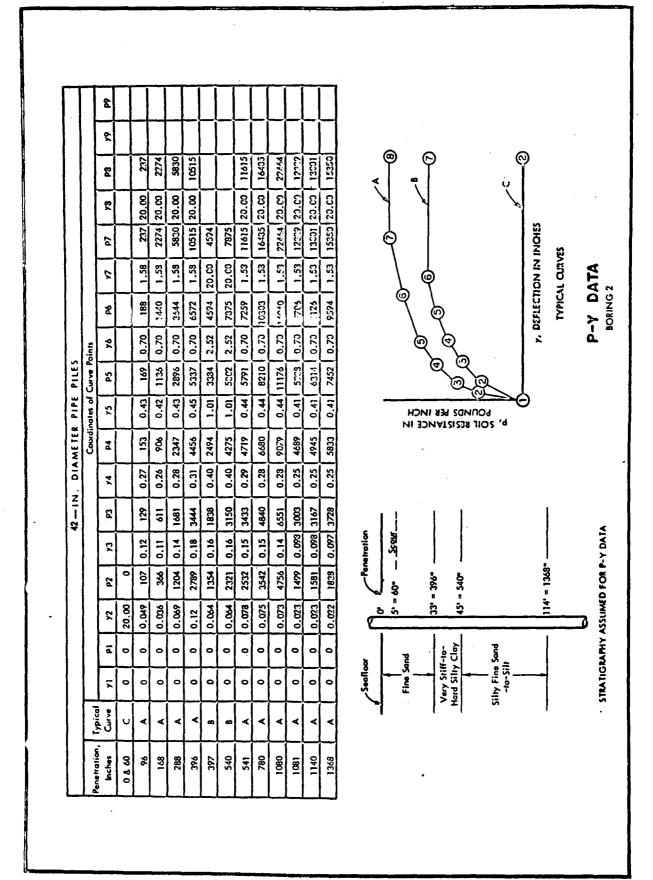
 $f_{b_{R/ing}} = \frac{\omega l^2}{25} = \frac{0.014 \, \%_n \, (684 \, in)^2}{2 \, \times \, 2400 \, in^3} = 1.4 \, km$

fototal 23.4 ksi

 $f_{a_{70h}}^{2} = \frac{454^{K}}{251 \text{ in}^{2}} + \frac{0.084^{5/2}(684)}{251 \text{ in}^{2}} = 2.0 \text{ ksc}$

ftotal = 22.0 + 1.4 + 2.0 = 25.4 ksi < 30 ksi

华口人

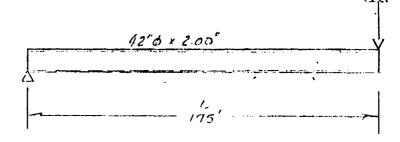


Sheet ___ _ of _ _ _

By J. Talbot Client U.S. Novy __ Subject Design of 23' MIN Strusture.

Date 8-23-76 Job No. _ 22-771-96 _ Calculation _ Pile Prolysis ____

Check Moximum Length of Pile for Fick-up-



$$f_b = \frac{M}{5} = \frac{3270 \ H \text{ tips x 17}}{5} = 16.3 \text{ kg}$$

16.3 ksi < 21.6 ksi .1.0K

SECTION 10.0

INSTALLATION ANALYSES

10.1 INTRODUCTION

This section contains the analyses considered pertinent to the installation of the structure.

Section 10.2 includes the check of the stresses on the structural members at the mudline due to the soil pressure on the jacket before the piling can be attached to the jacket.

Section 10.3 is the analysis of the recommended lifting eyes for the jacket lift. For the analysis of the recommended lifting eyes for the superstructure, refer to Report No. 27-771-98.

Section 10.4 is the lift analysis for the jacket. The computer output for the analysis is in Appendix B.8. For the lift analysis of the superstructure, refer to Report No. 27-771-98.

Section 10.5 contains the floatation analysis for the jacket.

Reference Drawings:

3016290 Jacket - Elevations
3016291 Jacket - Plan at El. (+) 12'-0"
3016292 Jacket - Plan at El. (-) 12'-0" & (-) 39'-0"
3016293 Jacket - Plan at El. (-) 66'-0" & (-) 93'-0"

3016297 Jacket - Lift Eye Details

10.62

CREST OFFSHORE, INC.

Dato 9-1-76 Job No. 27-221-25 Calculation LUSTAKATION PRINCESS

10.2 Sou PRESSURE ON STRUCTURE

Wright OF JACKET = 358.86 WEIGHT OF 1 PHF (163) = 139.2" BUDYANT HEIGHT OF JACKET = 153.61"

WIFIGHT = 358.86 + 139.8 - 153.61 = 338.45 BEARING AREA OF BRACES = 58917.6 BEARING STRESS ON BRACE = 350.45 = 5.7 1/w2 CHECK BENDING STRESS IN BRACES

> ERRCE: 20"d x 0.625 HALL l= 30.3073" W=1.38 x 30.3073'= 41.82 x

MMAX = HR = 158.41K

BRACE STRESS = 158.4x12 = 10.64 KS/ (PINNED

BRACE: 14" X 0.375 WALL 1 = 30.3073 N = 0.958 x 30.3073 = 29.02

MMAX = 109.95 K FIXED ENDS MAX 12 = 73.31

BRACE STRESS = 109.95x12 = 24.8 (PINNED)

10,03

By SECO. Client U.S. NAVY _ Subject DESIGN DE 23 MIN STRUCTURE Date 2-1-26 Job No. 27-221-25 Colculation ASTRUCTURE AMEXSIS

CHECK BENDING STRESS IN BRACES CONTO.

BRACE STRESS = 73.8 x 17 = 16.53 ESI (FIXED)

CREST	OFFSHORE,	INC.
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Subject AFS SA 25 23 MA TEU. By _2 12 13 Client 1 5. 11 10 V Date 2 = 1 = 76 Job No. 22 - 27 - 25

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ATIVE LENGTH AND SECTION FACTORS

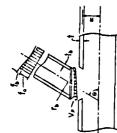
Q, is a design factor for the brace to chord diameter ratio, B.

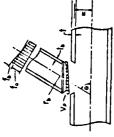
Q, is a design factor for the presence of axial load in the chord.
Q, = 1.0 for A < 0.44
Q, = 1.22 - 0.5A for A > 0.44 $Q_s = \frac{0.3}{\beta (1-833\beta)}$ for $\beta > 0.6$

Where A is the AISC ratio for the chord, i.e. # **Y**

J 14.	10		>	
60° 9 30°	•06			0 X
	١			
	-	IN S		NO SHEAR VP :
		90		

. 2	27-23	7:5	<u> </u>	Calculation	LUSTALLATION	LINALYSIS
3)LTS	Ş	6.57	6.57		
ox e	RESULTS	νp	2:11	0.80		
°C3	WOLE WILES	, O	C	0:		
-0.6 0	PUNCHING SHEAR ALLOWABLE PARAMETERS	ည	7.0	7.0		
	Shēlr Ers	Ks	<u></u>	7.93		
	UNCHING SHEA PARAMETERS	82	1:/	8521		
	PUNC PAR	0	C.B.	0,357 0,655 1,521 46,2 1,233 1,53		
	GEOMETRY PARAMETERS	J R/1	13.21	13,21		
		0 75/R	0152 0.539	00 00 00 00 00 00 00 00 00 00 00 00 00		
	GE C FAR	r \$/4	27/0	0.257		
		U. C.]			
Vp : 7 (5:N 0 12 + 12)	сновр	Llombor Size	245X0.875	24,5% 0.875		
	BRACE	fb (ksi)	3.53	1		
		fa (kai)	7	1.40		
		flombar Size	4/2X0.375	16,2×0.75		
•		ر 2ء دم.				
		ري ب:				





 $V_{\nu} = Q_{\nu}Q_{\ell} \frac{F_{\nu}}{c^{\alpha}} \frac{(plus~34)}{increase} \cdots (23)$ applicable)

 $Q_{\nu} = 1.0 \text{ for } \beta < 0.6$

F, = the yield strength of the chord member at the joint (or % the tenule strength if less)

 $\gamma = R/t$, the chord radius divided by the chord thickness.

GIVEN THE JOINT GEGIETAY AS DEFINED BY T, F, 0 AND 6 AS WELL AS ORDER STREETS F, AND F, THE FOLLOWING MAY BE USED TO DETERMINE THE CALCULATED PUNCHING THEAR UP:

By L. Talbet Client_U.S. Naky __ Subject_ Design of 23 MLK Structure_
Doto B-1Z-76 Job No. _ 27-771 - 25 Colculation _ LIFTING EVE ANALYSIS _

10.3 Lifting Eyes - Jacket

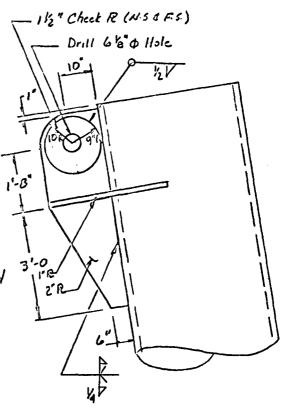
Vertical Lift

Weight of Jacket = 380K (excluding boot landing & bumpers)

Assumptions.

- 1. Entire weight is at one lift eye.
- 2. Impact Factor of 2.0.
- 3. Total applied load can be acting completely horizontal or completely vertical.
- 4. Sling 0 = 60° for maximum load

.. Max P = 880 t at 0 = 60°



Check Shear in Pin

Use 6.00" Pin in double shear

$$f_s = P = \frac{880^{\kappa}}{A} = 15.6 \text{ ksc}$$

Fs = 0.4 (36 Esi) x 1.33 = 19.2 Esi

15.6 4 19.2

Sheat __ _ of _ _ _

By N. Talkat Client U.S. Navy __ Subject Design of 33'MIN Structure __
Date 8 = 17-76 Job No. 27-771-95 Calculation _ LIFTIMG FYE ANALYSIS_

Lifting Eyes - Vacket

Vertical Lift

Check Bearing on Plate

$$f_{br} = \frac{P}{Dt} = \frac{880^{k}}{(6.00)(5.00)} = 29.3 \text{ ksi}$$

For = 0.9 (36 ksi) = 32.4 ksi

29.3 = 52.4

Check Pin Shearing Through Plates

$$A = 9[(9-3) \times 1.5] + 2[(10-3) \times 2]$$

$$A = 36 + 28 = 64 \text{ m}^2$$

13.8 < 19.2

Lifting Eyes - Jacket

Vertical Lift

Check Tension Through Lift Eye

13.8 4 28.7

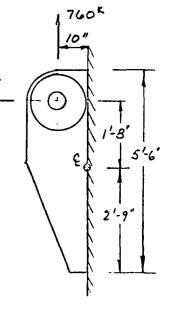
Check Combined Bending and Tension

$$f_s = \frac{760^k}{(66^n)(2^n)} = 5.8 \text{ ksi}$$

$$f_n = \frac{7600(6)}{2"(66")^2} + \frac{760"}{(66")(2")} = 11.0 \text{ ksc}$$

Using Mohr's Circle,

$$f_{n,max} = \frac{11.0}{2} + \left[\left(\frac{11.0}{2} \right)^2 + \left(5.8 \right)^2 \right]^{\frac{1}{2}} = 13.5 \text{ ksc}$$



15.57

By J_ Talkat client_ U.S_ Mary _ subject Design of 23 MLW Structure ___
Date_B=17-76 Job No. _ 27-771=95 _ calculation _ 4 ETI NIB EXE ANNUSIS

Lifting Eyes - Vacket

Vertical Lift

Assuming an average shear distribution, $f_{3 \text{ max}} = \left[\left(11.0 \right)^2 + \left(5.8 \right)^2 \right]^{\frac{1}{2}} = 8.0 \text{ Esc}$

F3 = 0.1 (36 ksc) × 1.33 = 19.2 ksc > 8.0

Assume shear distribution parabolic, maximum shear at center of plate.

fs = 1.5 (5.8) = 8.7 ksi

 $f_n = \frac{760}{(66')(2.0'')} = 5.8 \text{ ksi}$

 $f_{smax} = \left[\left(\frac{5.8}{2} \right)^2 + \left(8.7 \right)^2 \right]^{V_L} = 9.2 \text{ ks.}$

9.2 6 19.2

Check Weld of Check Plates

 $A_{R} = 1.5 = 0.30$

Pshear = 880 × 0.30 = 264 ×

 $\frac{P}{C} = \frac{264^{k}}{77.18''} = 4.7^{k}/m$

 $\omega = \frac{4.7}{11.2} = 0.42 \text{ in } Use \frac{1}{2}$ " fillet Weld

CREST OFFSHORE, INC.

Sheet ______of ____

By STITUS Ctient (1.5. NAVY ____ subject_DESIGN OF DESIGN OF DE

10.4 CIFT AMPLYSIS

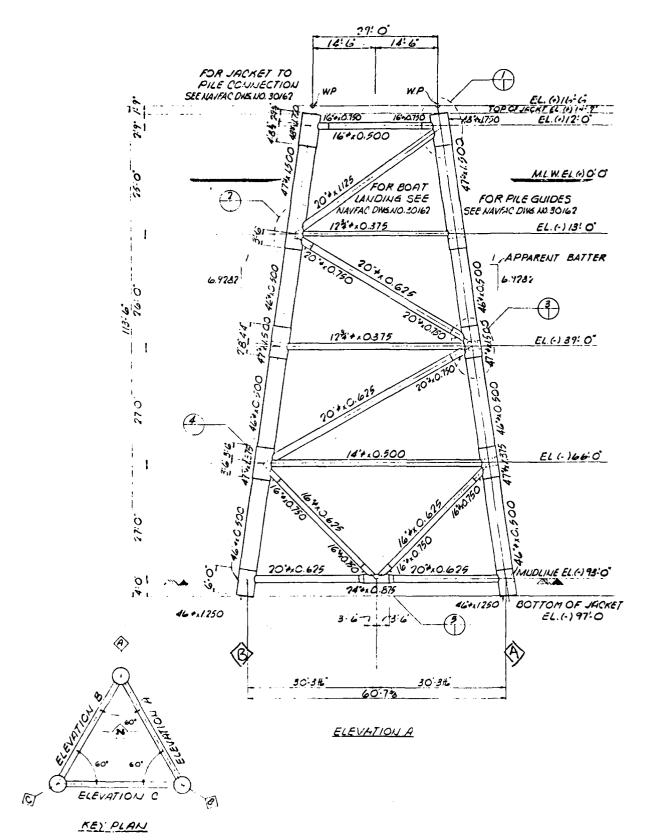
DESIGN CALCULATIONS 93' MLW STRUCTURE EAST COAST AIR COMBRT MANEUVERING R. . (U) CREST ENGINEERING IMC TULSA OK SEP 76 27-71-95 CHES/NAVFAC-FPO-7614 F/G 13/13 3/7 AD-A165 689 NĽ UNCLASSIFIED



MICROCOPY RESOLUTION TEST CHART

Sheet __ _ of _ _

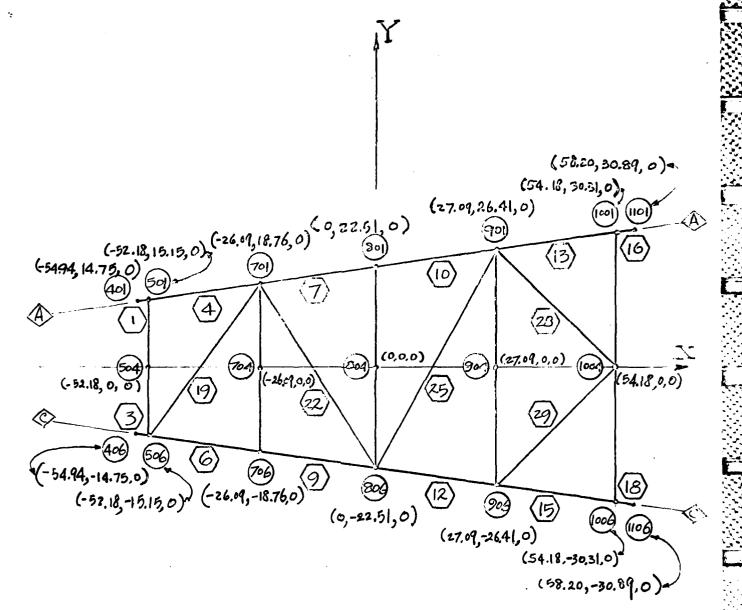
By C. Cherry Client U.S. MAYY __ Subject Lifting Analysis _____
Date & 123-76 Job No. 27-771-91 _ Calculation Platform Section ______



: · · ·

Sheet __ _ of _ _ _

By C. Charn Client U.S. MAYY _ subject Lifting Analysis _ ____ Date 8 = 20 = 76 Job No. 27 - 771-0 L _ calculation Plantown 2



$$27 \times \frac{\sqrt{37}}{6} \times \frac{6.9282}{7} = 27.09$$

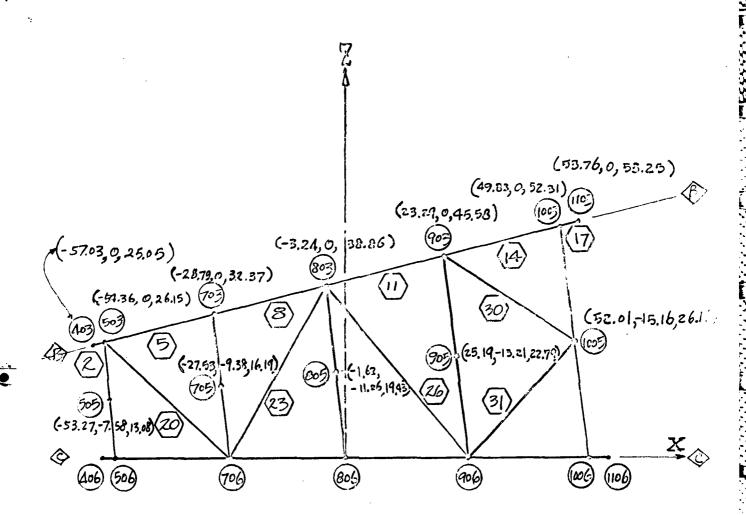
$$58 \times \frac{\sqrt{57}}{6} \times \frac{6.9282}{7} = 58.20$$

$$26 \times \frac{\sqrt{37}}{6} \times \frac{6.9282}{7} = 26.09$$

$$54.75 \times \frac{6.9282}{7} = 54.94$$

Sheet __ _ of _ _ _ _

By C. Cherr Client U.S. NAVY __ Subject Lifting Analysis _____ Date 8-20-76 Job No. 27-721-01 _ Calculation Platform #2 _____



$$\frac{\text{Joint}}{703}$$
 32.48 x $\frac{12}{\sqrt{145}}$ = 32.37

$$32.48 \times \frac{1}{\sqrt{145'}} = 2.70$$

$$\frac{503}{100}$$
 $\frac{26.24}{100}$ $\frac{12}{100}$ = 26.15

$$26.24 \times \frac{1}{\sqrt{145'}} = 2.18$$

$$\frac{403}{\sqrt{115'}} = 25.05$$

$$\frac{1003}{52.49} \times \frac{12}{\sqrt{145}} = 52.31$$

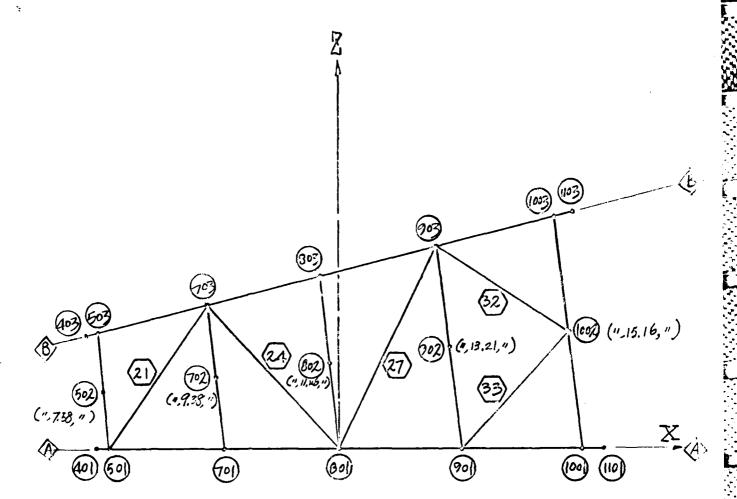
$$52.49 \times \frac{1}{\sqrt{145}} = 4.35$$

$$\frac{903}{45.74} \times \frac{12}{\sqrt{145}} = 45.58$$

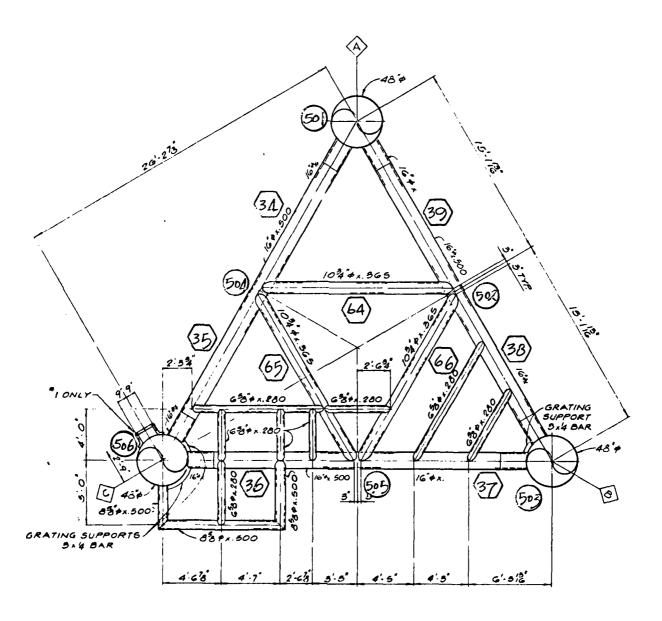
$$38.99 \times \frac{1}{\sqrt{145'}} = 3.24$$

Sheet __ _ of _ _ _

By C. Chern client U.S. NAVY _ subject Lifting Analysis _ ____
Date 8-20-76 Job No. 27-771-01 _ Calculation Platform 12 _ ____

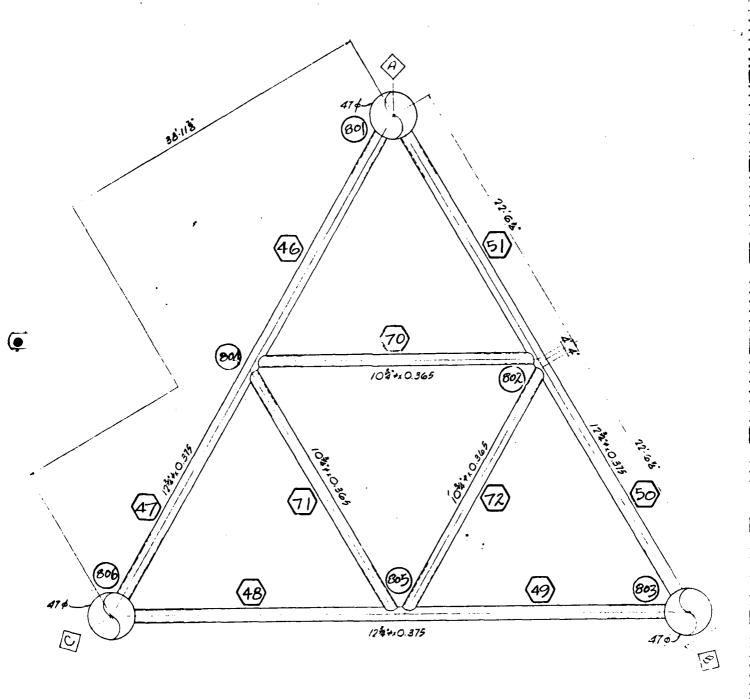


Sheet __ _ of _ _

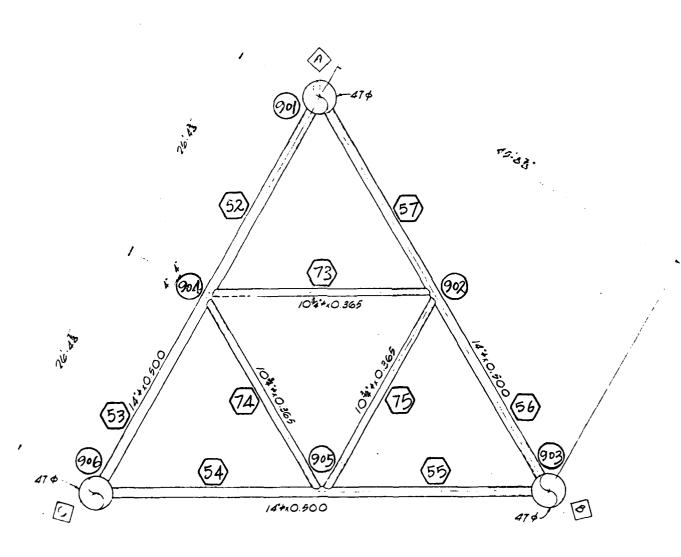


(A) (S)

By C. Cheyn client U.S. NAVY _ subject Lifting Analysis _ ____
Date B-22-76 Job No. 27-771-01 _ calculation Play form \$2 _ ____

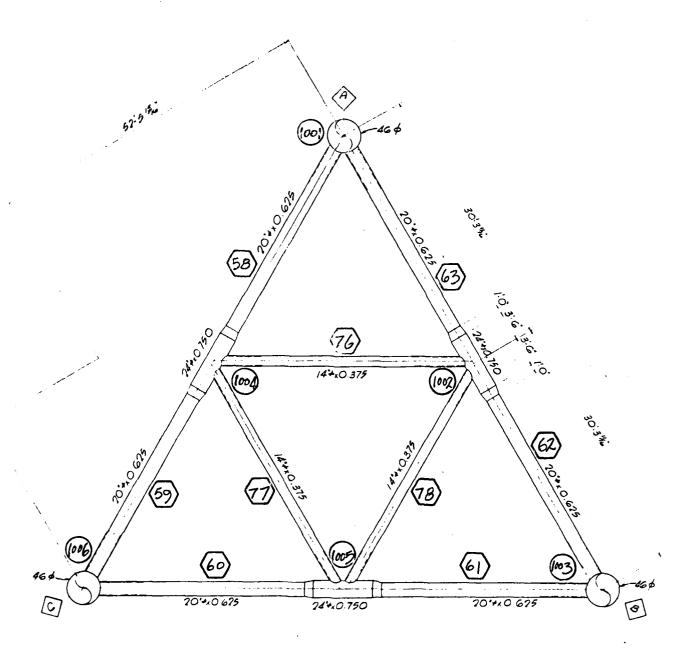


By C. Chern Client U.S. NAVY __ subject _ Lifting Analysis __ Date 0-23-76 Job No. 27-171-21 _ calculation Platform 172 _ _ _



•

By C. Charn client U.S. NAVY __ Subject __ Date 8-23-76 Job No. 27- ZZI-01 _ Calculation

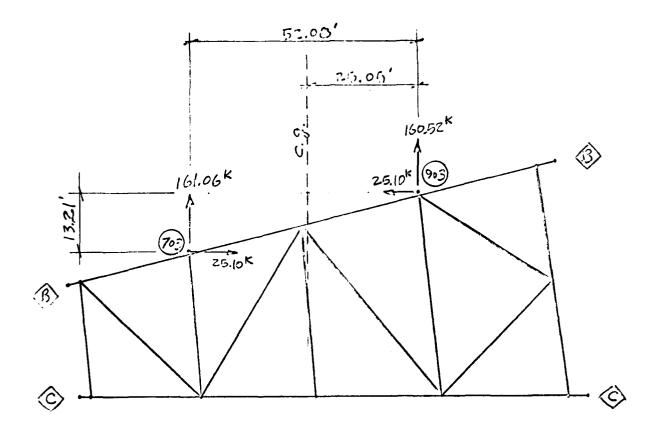


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Sheet __ _ of

By C. Chern Client U.S. NAUY __ Subject Lifting Analysis _____
Date 8-24-76 Job No. 27-771-01 _ Calculation Platform

LOCATION OF CENTER OF GRAVITY



$$\bar{\chi} = \frac{161.06 \times 52.08 - 25.10 \times 13.21}{161.06 + 160.52}$$

$$= 25.05'$$

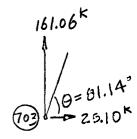
$$(25'-0\frac{1}{2}")$$

10,19

Sheet __ _ of _

By C. Chern Client U.S. NAVY __ subject Lifting Analysis _____
Date 8-24-76 Job No. 27-271-21_ calculation Plantorn #2-____

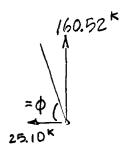
JOINT NO. 703



$$\theta = \tan \frac{161.06}{7.5.10} = 81.14^{\circ}$$

5ling Force = 160.06 5 in 81.14° = 162 kips

JOINT NO. 903

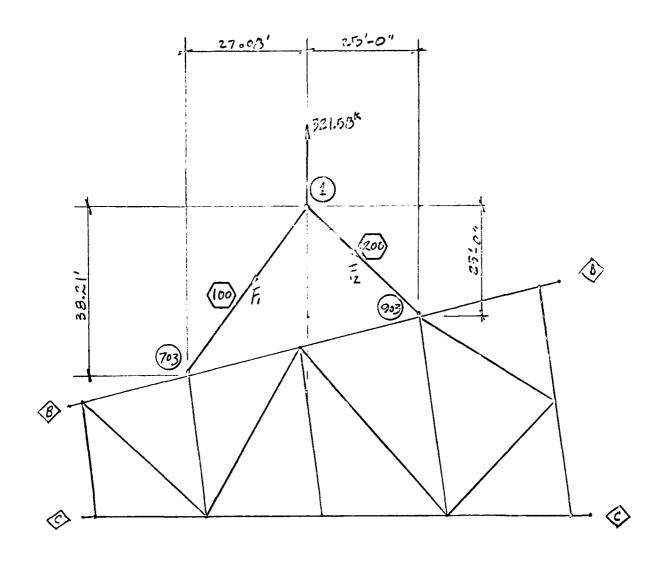


$$\phi = \tan \frac{1160.52}{25.10} = 81.12^{\circ}$$

/0. 20 Sheet _ _ of _ _ _

By C. Chern Client U.S. NAVY _ subject Lifting Analysis _____ Date 8-24-76 Job No. 27-771-01 _ calculation Platform #2_____

SINGLE POINT LIFTING (FOR CHECKING DILY)



Sling Length "200
$$l_2 = \sqrt{25^2 + 25^2} = 35.36'$$
 (35'-4")

"100 $l_1 = \sqrt{27.08 + 38.71^2}$

$$= 46.83'$$
 (46'-10")

10,21

Shcot __ _ of _

By C. Charn Client U. S. NAVY __ subject Lifting Analysis _____
Date 3-24-76 Job No. 27-221-0L _ calculation Plate From #2_____

Equilibrium @ Pt. 1

$$\left(\frac{F_{2}}{\sqrt{2'}} + F_{1} \times \frac{38.21}{\sqrt{27.03^{2} + 38.21^{2'}}} = 3.21.58\right) \tag{9}$$

$$\frac{F_2}{\sqrt{27}} = F_1 \times \frac{27.08}{\sqrt{27.08^2 + 38.21^2}}$$
 (b)

from Eq.(a)
$$\frac{(27.08 + 38.21)F_1}{\sqrt{27.08^2 + 38.21^2}} = 321.58$$

$$F_1 = \frac{321.58 \times 46.33}{65.29}$$

$$F_2 = 230.66 \times \frac{27.08\sqrt{2}}{46.83}$$

10.22

By ADDR Client U.S. NAYY Subject DESIGN DE 93' MUNSTRUCTION DATE 2-3-76 Job No. 27-271-25 Calculation FLORIDITION DATE SIS _

10.5 FLOATATION ANNIYSIS

Buoyancy of lacket in Launch Mode = 414.8 K (Ref. Section 10.5, p 10.23)

Weight of Jacket in Launch Mode Material Listing Anodes

= 359.5 K = 10.9 K

370.4 K

414.8"> 370.4"

:. Floatation is acheived

. . .

Sheat	_	_	_	of	 _	

By L. KIRK Client U.S. NAVY _ Subject DESIGN OF 93'MLW STRUCTURE Date 9-1-76 Job No. 27-771-95 _ Calculation BOUYANCY - LAUNCH

		VOLUME	
DESCRIPTION	QTY.	SEA WATER	
		DISPLACED	
48.0"\$x 1.75 WT	22.50 FT	282.60 FT3	
47.5" × 1.50 WT	114.15 FT	1404.05 FT3	
47.0" x 1.25 WT	18.00FT	216.72 FT3	
45.5" × 0.50 WT	183.99 FT	2077.25 Ft3	
24.0" × 0.875WT	21.00 FT	65.94F73	
20.0" \$ 1.125WT	114.45 FT	249.50Ft3	
20.0" × 0.75 WT	26.50FT	57.77 Ft3	
20.0" × 0.625 WT	449.10FT	979.04 Ft3	_1
16.0" \$ x 0.75 WT	52.75Ft	73.32 Fτ3	
16.0"\$ x 0.625 WT	212.70FT	295.65 FT3	ا ا
16.0" × 0.50WT	80.91 FT	(12.46 F73	ob
14.0"\$ x 0.50WT	147.96FT	156.84F73	(
(4.0"\$ x 0.375WT	85,50FT	90.63F73	
12.75" \$x 0.375 WT	225.36FT	198.32Ft3	3
10.75" \$x 0.365w7	240.00FT	151.20 F73	97)
8.625" × 0.50 WT	18.42 FT	7.37 FT3	3
6.615"\$x0.280wt	54.75FT	12.59 FT3	J
3.50" × 0.216 WT	24.75FT	1.49 Ft3	3
2.875"×0.315w	6.00F7	0.24 Ft3	<i>3</i>
2.375" × 0.154 WT	310.50FT	9.32 Ft3	3
2.00 STL. PLATE	45.84F72	7.66 Ft3	칠
1.50 STL. PLATE	17.69 FT2	2.21 FT3	Me.
1.00 STL. PLATE	45°51 645	3.76FT3	88
0.75 STL. PLATE	47.10 Ft2	2.94Ft3	2
0.625 STL. PLATE	37.68Ft2	1.96 FT3	\cup
O.SO STL. PLATE	64.50 Ft2	2.69 Ft3	
O.MS STL. PLATE	4.13F72	0.13 FT3	
0.250 STL. PLATE	1.70 Ft2	0.04FT3	
GRATING	196.28 Ft2	16.29 Fr3	
AUGLE 4"x6"x 3/8"	24.00 FT	1.25 F73	
	TOTAL	6481.23FT3	

(Bouravey) Weight of Sea Water Displaced = 6481.23 × 64 = 414798.72#

OFFSHORE, INC. **CREST**

(

10.24 -- of ---

By L. KIRK Client U.S. NAVY __ Subject DESIGN OF 93'MLW STRUCTURE Date 9-1-76 Job No. 27-771-95 Calculation BOUYANCY - IN PLACE

		VOLUME	1.) FLOODED I.D.	
DESCRIPTION	QTY.	SEA WATER	02	!
= 5 = 1,4,1		DISPLACED	2.) ABOVE WATER LINE	
48.0"\$x 1.75 WT	22.50 FT	282.60 F73	- 282.60Ft3(2.)	JACKET
47.5" × 1.50 WT	114.15 FT	1404.05 FT3	- 1404.05F73(1.42)	
47.0" x 1.25 WT	18.00FT	216.72 FT3	194.41873 (1.)	LEGS
45.5" × 0.50 WT	183.99 FT	2077.25 Ft3	- 1987.20Ft3(1.)	
24.0" × 0.875WT	21.00 FT	65.94Ft3		
20.0" \$x 1.125WT	114.45FT	249.50Ft3		
20.0" × 0.75 WT	26.50FT	57.77 Ft3		
20.0" × 0.625 WT	449.10FT	979.04 Ft3		
16.0" \$ x 0.75 WT	52.75Ft	73.32 FT3	- 20,59F+3(2.)	(+)15,-0,,
16.0"\$ x 0.625 WT	212.70FT	295.65 Ft3		ELEV.
16.0" × 0.50WT	80.91 FT	112.46 FT3	112.46F73(2.)	
14.0"\$ x 0.50WT	147.96FT	156.84Fz3		
14.0" × 0.375WT	85.50FT	90.63773		
12.75" \$x 0.315 WT	225.36FT	198,32Ft ³		
10.75" \$x 0.365wT	240.∞Ft	151.20 Ft3	- 28.05Ft3(2.)	(+) 15,0,
8.625" × 0.50 WT	18.42FT	7.37 FT3	7.47 Ft3 (2.)	ELEV.
6.625" \$x0.280 WT	54.75FT	12.59 FT3	13.(1 Ft3 (2.)	
3.50" × 0.216 WT	24.75FT	1.49 Ft3		
2.875"\$x0.315w	6.00F7	0.24Ft3		
2.375" × 0.154 WT	310.50FT	9.32 Ft3		
2.00 STL. PLATE	45.84F72	7.66 Ft3	- 7.66 Ft3 (2.)	LIFTING
1.50 STL. PLATE	17.69 FT2	2.21 FT3	2.21 Ft3 (2.)	EYES
1.00 STL. PLATE	45.27 6-13	3.76 FT3	3.76Ft3 (2.)	
0.75 STL. PLATE	47.10 Ft2	2.94573		
0.625 STL. PLATE	37.68Ft2	1.96 F73		
O.SO STL. PLATE	64.50 Ft2	2.69 Ft3		
O.MS STL. PLATE	4.13F72	0.13 FT3		
0.250 STL. PLATE	1.70 FT2	0.04FT3		(1)
GRATING	196.28 Ft2	1	- 16.29 F73(2.)	Į.
ANGLE 4"x6"x 3/8"	24.00 FT	1.25F73	1.25FT3(2.)	ELEV.
	<u> </u>	,		ا
	TOTAL	6481.23FT	4081.11F73	

(Bouyaney) Weight of Sea Water Displaced = (6481.23)-(4081.11) x 64=153607.68#

SECTION 11.0 CORROSION PROTECTION

11.1 INTRODUCTION

The surface area of a marine structure is divided into three zones for corrosion protection consideration, the Submerged Zone, the Splash Zone, and the Atmospheric Zone.

The Submerged Zone is protected from corrosion by cathodic protection through the use of sacrificial anodes. The Splash Zone is protected by using one half inch thick extra material in excess of that needed for strength and then painted. The Atmosphere Zone is protected with paint.

11.2 DESIGN DATA

Zones for Corrosion Protection:

- 1. Submerged Zone El. (-) 4.0 feet to El. (-) 93.0 ft.
- 2. Splash Zone El. (+) 11.0 feet to El. (-) 4.0 feet.
- 3. Atmospheric Zone- El. (+) 75.0 feet to El. (+) 11.0 feet.

Current Requirements:

Current Density = 6 MA/ft^2 of surface in water 2 MA/ft^2 of surface in mud

Design Life:

N = 20 years

CREST OFFSHORE, INC.

Sheet _ _ _ of _ _ _ _

By L. KIRK Client U.S. MAYY _ Subject DESIGN OF 93' MLW STRUCTURE Date 7-4-76 _ Job No. 27-771-95 _ Calculation SURFACE AREA CALCULATION _

Location	Description	QTY.	SURFACE AREA (F72)	(FT2) TOTAL SURFACE AREA
-4'-0" To-13'-0"	47" \$ x 121-6" SL	3	(53.3	461.4
i to in	20"\$ x 151-4" DB	3	80.3	240.9
-131-0" (PLAN)	123/4" \$ x 37-6" HB	3	125.3	375.8
10 10 10	103/4" × 18'-9" HB	3	52.7	(53.1
- 131-01, Lo-3210"	46"\$ x 18'-2" JL	3	213.7	656.1
ic is is	17.10 x 31-01 7r	3	36.1	258.3
lt 11 12	20" \$ x 481-4" DB	3	253.2	759.6
-391-04 (PLAN)	123/4" \$ x 45 - 1" HB	_3	150.5	451.5
u u u	103/1/10 x 221-6" HB	3	63.2	189.6
-391-0" To-66-0"	46" \$x 20'-10" SL	3	250,8	752.4
u v u	47" \$ x 7'-0" JL	3	86.1	252,3
ic ic ic	20"4x 56'-0" DB	3	29304	830.2
-66-0" (PLAN)	14"\$ x 521-10" HB	3	193.9	581.6
te ic ic	(03/4" \$ x 26'-5" HB	3	74.2	222.6
-66-0"70-93-0"	46" \$x 211-6" JL	3	253.9	77607
huu	47" px 6'-0" JL	3	73.8	221.4
uuu	16" \$x 381-0" DB	6	(59.6	957.6
	20" \$x 271-3" HB	6	142.7	856.2
is a H	24" \$x 6'-0" HO	3	37.6	112.8
le n u	14" \$ x 30'-3" HB	3	111.0	333.0
		ļ		9504.1
PILING	42" \$ x 2.00WT x 270'-0"	3	2968.8	+8906.4
	,0-	·		18410.5
		<u> </u>		
	,			
		<u> </u>		<u> </u>

70

CREST OFFSHORE, INC.

11.04

By ADD Client U.S. NAYY Subject DESIGN OF 93' IMIN STRUCTURE

Date 9-3-26 Job No. 22-271-25 Calculation CORROSION PROTECTION

Total Current Requirements

I = 6 mg × 9504 F2 + 2 mg × 6000 F2

I = 74.8 Amps

Capacity of Allox

٦

Use Alluminum - Zinc - Mercury Alloy

C = 1250 amp-hes

Total Weight of Sperificial Anodos

Wt = I × N × 8760

Wt = 74.8 x 20 x 8760

Wt = 10484 16s.

Using 725# Anode n = 10484 = 14.46

USE 15 @ 725# Anode

11.4 SPLASH ZONE

The Splash Zone is protected by first using one half inch thick extra material in excess of that needed for strength, and then by applying paint to the structural members in the zone.

11.5 ATMOSPHERIC ZONE

The Atmospheric Zone is protected by paint. The surface area of the structure requiring paint is 8,500 square feet. The surface area calculations can be found in Report No. 37-771-98, Section 2.7, Paint Area.

SECTION 12.0
MATERIAL LIST AND WEIGHT

12.0 INTRODUCTION

This section includes a material listing and total weight of each major component of the structure including the superstructure, jacket, boat landing, boat fenders, and piling. The material listing in this section is a summary and includes only the total length and weight of each particular shape for each of the major components. A more detailed listing is found in the computer output in Appendix B.9.

CREST OFFSHORE, INC.

By FEDD. Client U.S. MAYY __ Subject DESIGN DE 93 MIN _TRUE IR.

Date 2-3-76 Job No. 27-771-25 Calculation MATERIAL USTIME & WEST.

12.2 Material Listing ran Weight - Superstructure

BILL OF MATZRIALS DU U.S.NAVY AGER PLAT NOMINAL DIMENSION 30,000 0.0. X 1,750 30,000 0.0. X 1,750 6,625 0.0. X 0,300 6,625 0.0. X 0,300	3UMMARY SUPERSTRUCTURE 2 ON TOTAL LENGTH SO WT ON WT ON WT ON WT SO WT ON WT SO WT ON WT SO WT ON WT O	7771e01 BILL OF MATERIALS & WEIGHT (PDUND) 657443 792739 1907058 1005.00 2176.58 9158.79 2410.29 788.14 949.43 1237.31 318.59 316.12	
### ### ##############################	SUPERSTRUCTURE TOTAL LENGTH (FEET) 15,00 15,00 15,00 15,00 15,00 15,00 15,00 15,00 10,00 10,00 10,00 10,00 10,0	7271=01 BILL OF MATERIALS TOTAL WEIGHT (PDUND) 1927448 1927448 1927448 1905500 2105500 2105500 2105500 2105500 2105500 2105500 2105500 2105500 2105500 2105500 2105500 210512	
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######################################		* * *	50.6	Se S	
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CREST	OFFSHORE,	ITIC.	Sheat of
By 2 3	Client 1/2 Job No. 22	Marie	Subject Laster 27 20 1/1/2 200 1/2 Calculation _ Matter Calculation _ Ma
<u> 12. 3</u>	Material	Listing	and Weight - Inchate

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CREST OFFSHORE, INC.

BADER Client U.S. Navy ___ Subject ASSIGN OF 93/11/1/ ETRACTURED

Date 2-3-26 Job No. 22-221-25 __ Calculation Lateral Lateral

12.4 Material Listing and Weight - Bank Landing

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	27-771-01	
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ANGLE			
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12.10 U/E/ 30A7 LANSING 274771401 BILL OF MATERIALS 23909,20 183 9,00 TOTAL WEIGHT 7,330 LBS PER 39 24 O,750 THICKNESS U,500 THICKNESS 9812445 PLATE 0 0 0 0.0 0 0.0 0

CREST OFFSHORE, INC.

BY ALLEY Client ILS MAIN _ Subject DESIGN & BE MINISTRUST DESIGN & BE MINISTRUST DESIGN & BE MINISTRUST DESIGN & BETWEET COLUMN DESIGN & BETWEET DESIGN & CONTRACTOR DESIGN & BOOK FORDERS

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CREST OFFSHORE, INC.

By ADLOR. Client U.S. NAYY __ Subject DESIGN OF 23' MILL STRUCTURE
Date 9-3-76 Job No. 22-771-95_ Calculation MATERIAL LISTING & Material Listing and Weight - Piling

•	2 27-771-01 BILL OF MATERIALS	(POUND)	6333,89 238431,56 795345,38	12547.94	1064323.00 LBS					•	
	JACKET PILING SITE	TOTAL LENGTH (FEFT)	\$17.00 \$37.00	00°807	L MEIGHT						
	BILL OF MATERIALS SUMMARY. U.S.NAVY ACMR PLATFORM	NOMINAL DIMENSION	42,000 0,0 x 2,500 H 42,000 0,0 x 2,575 H 42,000 0,0 x 2,000 H	0.0 x 0.625	TOTAL						

APPENDIX A.1
ENVIRONMENTAL DATA

TABLE 18: 50 YEAR STORM WIND, TIDE, AND WAVE CHARACTERISTICS: 36°13'36.3"N, 75°14'59.6"W: SPECIFIED 93 FOOT CHART DEPTH: OFFSHORE KITTY HAWK, NORTH CAROLINA

Chart Depth	93.0 Ft.
Highest Astronomical Tide	4.5 Ft.
Storm Tide	3.6 Ft.
Total Tide	8.1 Ft.
Still Water Depth	101.1 Ft.
Height of Maximum Wave	60.8 Ft.
Period of Maximum Wave	13.6 Sec.
Crest Elevation Of Maximum Wave Above Still Water Level	43.6 Ft.
Crest Elevation of Maximum Wave Above Chart Datum	51.7 Ft.
Crest Elevation of Maximum Wave Above Bottom	144.7 Ft.
Length of Maximum Wave	783.5 Ft.
I Hour Wind	114 Mph
0.5 Hour Wind	120 Mph
l Minute Wind	145 Mph
Maximum Instantaneous Gust	174 Mph

A. H. GLENN AND ASSOCIATES -

TABLE 19: 50 YEAR COMBINED WIND DRIFT, DENSITY, AND TIDAL CURRENT VERSUS PERCENT OF DEPTH: 36°13'36.3"N, 75°14'59.6"W: SPECIFIED 93 FOOT CHART DEPTH: OFFSHORE KITTY HAWK, NORTH CAROLINA

Percent Of Depth	Current Speed (Ft/Sec)
0%	4.5
10%	4.2
20%	3.8
30 %	3.5
40%	3.2
50 %	2.9
60 %	2.6
70%	2.2
80%	1.9
90%	1.6
100%	0.0

APPENDIX A.2
WAVE PROFILES

FIELD VARIABLES FOR MAVE: CREST: 93 FT. DEPTH

MAVE PARAMETERS:

4.50 FT./SEC. HEIGHT = 60.80 FT., PERIOD = 13.60 SECS., LENGTH = 829.12 FT. 1.30 FT./SEC., SURFACE CURRENT = 3.00 FT. BOTTOM CURRENT # PILF DIAMETERS

FTFECTS SURFACE TREE 100 TNCLUBES MODIFICATIONS

日の日 PRESSURES IN ALL L

WAVEL CREST: 93 FT. DEPTH

STATE OF THE PROPERTY OF THE P

**************************************	-340.nn	-360.00	-340,00	-370.no	-300.00	-280.00	-260.00	-740.00	-270.00
SURFACE = ELEVATION (FT.)	86,74	86,52	A6.70	A7.17	A7.6A	A8,35	89.22	90.32	9 ₁ .73
SURFACE MOR. OHANG PRESSES HOR. OHANG PRESSES VERLEVAN AND PRESSES HOR. THEAST PRESSES VER. THEAST PRESSES	-12.27 -1.17 -1.17 -1.17 -1.17 -1.17 -1.17 -1.17 -1.17 -1.17 -1.17	-11.70 -2.20 -0.43 5.96 0.00 0.00	-10.01 -3.13 -0.00 0.00 0.00 0.00	-9.79 -4.77 -1.72 -9.76 0.00 0.00	*0.27 -6.59 -3.62 10.22 0.00 0.00 0.00	-6.43 -0.17 -5.01 12.09 0.00 0.00 0.00	-4.43 -10.53 14.70 0.00 0.00 0.00	-2.40 -13.69 -17.56 0.00 0.00 0.00	-0.67 -14.01 -19.70 21.15 0.00 0.00 0.00
HORE DRAG PRESS. HORE INER PRESS. VER. DRAG PRESS. VER. INER PRESS.	0.09 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00
HOR HORAG PPESS.	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
HOR INER PPESS.	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
VER INER PRESS.	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
VER INER PRESS.	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
MDQ. DRAG PRESS.	0.00	0.00	0.00	n.no	0.00	0.00	0.00	0.00	0.00
HDR. INEM PRESS.	0.00	0.00	0.00	n.no	0.00	0.00	0.00	0.00	0.00
VER. DRAG PRESS.	0.00	0.00	0.00	n.oo	0.00	0.00	0.00	0.00	0.00
VER. INER PRESS.	0.00	0.00	0.00	n.oo	0.00	0.00	0.00	0.00	0.00
HOR. ORAG PRESS.	0.00	0.00	0.00	0.00	0.00	0.00	0.00	-2.40	-0.67
HOR. INER PRESS.	0.00	0.00	0.00	0.00	0.00	0.00	0.00	-13.69	-16.01
VER. DRAG PRESS.	0.00	0.00	0.00	0.00	0.00	0.00	0.00	-12.75	-19.76
VER. INER PRESS.	0.00	0.00	0.00	0.00	0.00	0.00	0.00	17.96	21.15
ELEV 90.00 HOR. DRAG PPESS. HOR. INER PRESS. VER. ORAG PRESS. VER. 14FH PRESS.	-12.27 -1.59 -0.17 5.33	-11.70 -2.20 -0.43 5.96	-10.91 -3.18 -0.90 7.26	=9.79 =4.77 =1.72 8.76	-8.27 -6.89 -3.02 10.22	-6.43 -8.37 -5.01 12.04	-4.63 -10.51 -8.05 14.70	-7.39 -13.74 -12.67 17.69	-0.62 -18.22 -19.13 20.65
ELEY = AD.OO HOR. DRAG PPESS. HOR. INER PPESS. VER. DRAG PRESS. VER. INER PRESS.	-13.02 -1.69 -0.15 5.12	-12.40 -2.51 -0.39 5.71	-11.51 -3.64 -0.81 -6.81	-10,76 -5,26 -1,51 8,12	-R.60 -7.15 -2.63 9.48	-6.60 -9.21 -4.30 11.14	-4.39 -11.72 -6.81 13.30	-7.18 -15.09 -10.47 15.73	~0.45 ~19.31 ~15.58 17.91
HOR. DRAG PRESS.	-14.31	*13.57	-12.50	-11.04	-9.16	-6.93	-4.49	-2.12	-0.37
HOR. TIMEP PRESS.	-1.65	*2.91	-4.21	-5.88	-7.85	-10.09	-12.79	-16.15	-20.17
VER. ORAG PRESS.	-0.12	*0.32	-0.66	-1.23	-2.11	-3.46	-5.45	-8.33	-12.26
VER. INCH PRESS.	4.69	5.22	6.12	7.22	8.46	9.93	11.71	13.63	15.34
HOR. DRAG PRESS.	-15.75	-14.89	-13.65	-11.98	-9.86	-7.41	-4.74	-2.19	-0.36
WOR. INER PRESS.	-2.02	-3.24	-4.66	-6.42	-6.47	-10.84	-13.64	-16.97	-20.82
VER. DRAG PRESS.	-0.09	-0.25	-0.51	-0.94	-1.62	-2.64	-4.16	-6.32	-9.23
VER. INER PRESS.	4.17	4.63	5.36	6.29	7.37	8.62	10.08	11.59	12.90
HOR. DRAG PRESS.	-17.34	-16.38	-14.97	-13.10	-10.78	-8.07	-5.17	-2.40	-0.41
HOR. INEH PRESS.	-2.17	-3.52	-5.05	-6.88	-9.00	-11.46	-14.31	-17.61	-21.31
VER. DRAG PRESS.	-0.07	-0.18	-0.37	-0.66	-1.16	-1.89	-2.97	-4.51	-6.56
VER. INEH PRESS.	3.59	3.97	-56	-5.32	4.22	7.26	8.42	-59	10.56
ELEY ARIONO	-19.13	-14.06	-16.50	-14.43	-11.89	-9.94	-5.78	-2.77	-0.57
MOR. DHAG PRESS.	-2.30	-3.74	-5.37	-7.26	-9.44	-11.95	-14.84	-18.10	-21.66
VER. DHAG PRESS.	-0.04	-0.12	-0.24	-0.45	-0.76	-1.74	-1.95	-2.95	-4.47
VER. THER PRESS.	2.93	3.24	3.71	4.32	5.03	5.06	6.75	7.63	8.35
HOR. DRAG PRISS.	-21.13	-19.94	-18.25	-16.00	-13.23	-10.03	-6.61	-3.42	-0.89
HOR. INFP PRISS.	-2.40	-3.91	-5.60	-7.54	-9.78	-12.33	-15.23	-16.45	-21.90
VER. DRAG PRISS.	-0.03	-0.07	-0.14	-0.26	-0.44	-0.71	-1.11	-1.74	-2.81
VER. INFH PRISS.	2.24	2.47	2.92	1.28	3.61	4.42	3.07	5.70	6.21
HOR OPES PRICO HOR INEH PRISS VER DHAG PRISS VER INER PRISS	-27.36 -2.47 -0.71 1.51	*22.17 *4.03 *0.03 1.67	-20.25 -5.77 -0.06 1.90	-17.02 -7.75 -0.12 2.20	-14.83 -10.02 -0.20 2.56	-11.78 -12.59 -0.32 -2.93	-7.67 -15.50 -0.50 3.39	-4.23 -18.69 -0.79 3.80	-27.00 -1.33 4.11
HOR. DRAG PRISS.	-25.86	-24.51	-27.53	*19.93	-14.73	-13.07	-7.01	-5.16	*7.71
HOR. THEM PRISS.	-2.57	-4.11	-5.87	-7.87	-10.16	-12.75	-15.65	-18.62	*27.15
VER. DRAG, PRISS.	-3.00	-0.01	-0.02	*0.63	-0.05	-0.03	-0.12	-0.19	*0.35
VER. THEM PRISS.	0.74	0.84	-0.26	1.11	1.29	1.49	1.70	1.90	2.05
HOR. DWAS PORSS. HOR. THEN PRESS. VEH. DRAS PORSS. VER. THEN PRESS.	0.00 0.00 0.00 0.00	-7.13 -4.13 -6.69 -6.00	-1.97 -5.91 0.00 0.00	-1.75 -7.97 -0.00 -0.00	-1.49 -10.21 0.00 0.00	*1.17 *12.99 6.00 0.00	-0.04 -15.71 0.00	-0.59 -18.87 0.00 0.00	-0.24 -22.16 0.60 0.00

WAVE: CPEST: 93 FT. DEPTH

***** x = (FT.)	*****	-200-00	-180.00	-160.00	-140.nn	-120.00	-100.00	-80.00	-60.00	-40.00
SURFACE = ELEVATION	(FT.)	93.50	95.72	98,48	101.88	106.04	111,10	117.14	124.27	132,55
SURFACF HOR. DRA HOR. DRA VER. DRA VER. DRA HOR. DRA HOR. DRA VER. DRA VER. DRA VER. DRA	STATE OF STA	0.00 -29.71 -29.71 -29.71 -0.00 -0.00	78.77 741.27 76.47 0.000 0.000	6.05 -35.439 -61.39 -7.72 -0.00 -0.00	77.577 -64.54 -60.000 -000	51.36 -111.63 -27.00 -0.00 -0.00	40.53 -139.77 -20.82 -0.000 -0.000	215.36 -70.70 -162.96 11.37 0.00 0.00	155.46 -77.91 -171.74 -1.37 0.00 0.00 0.00	168.01 -87.34 -150.99 -20.99 168.01 -87.34 -150.99
ELEV # MOR. DRAI HDR. INF. VER. INF.		0.00 0.00 0.00 0.00		0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	155.46 -77.91 -171.74 -1.37	453.49 -78.14 -136.11 -23.40
HOR DRA- HOR DRA- VER DRA- VER INFO	120.00 G PRESS. G PRESS. G PRESS.	0.00 0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.07 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	40.50 -62.63 -139.77 -20.82	215.36 -70.70 -162.96 11.37	475.35 -73.18 -148.29 -5.01	639.45 -63.86 -91.68 -29.04
HOR. ORAL HOR. INCI VER. INCI VER. INCI	110.00 G PRESS. G PRESS. R PRESS.	0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	23,26 -43.97 -84.57 -29.49	51.36 -53.64 -111.63 -27.09	104.93 -62.17 -135.43 -20.02	188.94 -65.09 -130.55 6.39	303.28 -63.01 -105.12 -10.76	432.71 -52.61 -62.52 -30.63
HOR. INCI VER. INCI VER. INCI VER. INCI	100.00 G PRESS. G PRESS. G PRESS.	0.02 -23.03 -29.71 23.91	1.78 *28.62 *43.27 26.47	6.05 ~35.42 ~61.39 28.72	22.90 -43.67 -60.80 -81.75	57.15 =51.35 -95.10 22.94	100.60 -56.44 -101.04 13.72	171.98 -57.80 -95.07 1.33	262-65 -54-27 -74-40 -13-71	359.84 -43.75 -42.94 -29.93
HOR DRAG HOR INEI VER DRAG VER DRAG VER THE	90.00 PRESS. PRESS. PRESS.	0.04 -23.35 -27.69 -2.74	2.16 -28.97 -38.25 -24.16	9.25 -35.30 -50.45 -24.44	24.63 -41.99 -62.72 22.46	52.34 =47.80 =72.12 17.32	95.71 -51.27 -74.91 9.00	156 • 16 -51 • 32 -60 • 58 -1 • 96	230.61 -66.91 -52.41 -14.82	306.66 =36.78 =29.54 =27.96
HOR DRAI HOR INEI VER DRAI VER INEI	AD.OD PRESSS. R PRESSS. G PRESSS.	0.13 -24.11 -22.23 19.53	2.70 *29.31 *30.25 20.40	10.22 -34.83 -39.12 20.07	25.45 -40.23 -47.66 17.78	51.40 =44.56 =53.69 12.95	90.21 =46.70 =54.70 5.57	141-58 -45-73 -46-85 -3-92	202.71 =40.82 =36.58 =14.71	263.49 *31.27 *20.23 *25.30
HOR. INC. VER. DRAG VER. INC. VER. INC.	70.00 PRESS. PRESS. PRESS.	0.21 -24.65 -17.26 16.54	3.04 -29.39 -23.16 17.00	10.73 -34.17 -29.46 16.32	25.51 +36.52 +35.24 13.97	49.64 -41.66 -39.00 9.58	84.38 -47.73 -39.06 3.15	128.83 -40.99 -34.32 -4.90	179.11 -35.84 -25.11 -13.82	228,40 -26,93 -13,69 -22,30
HOR. DRAG HOR. INC VER. INC VER. INC	60.00 PRESS. PRESS. PRESS.	9.24 -25.01 -12.86 13.77	3.17 -20.31 -17.03 13.94	10.80 -33.43 -21.35 13.09	24.94 -36.92 -75.12 10.06	47.30 +39.12 +27.35 6.99	78.49 -39.36 -27.02 1.52	117 - 28 +37 - 05 -23 - 40 -5 - 15	159.79 -31.81 -16.87 -12.42	199.85 -23.52 -19.14
HOR. DRAI HOR. 15FI VER. DRAI VER. 18FI	50.00 PRESS. PRESS. PRESS.	0.71 -25.23 -9.07 11.15	3.09 *29.14 *11.86	10.49 -32.71 -14.67 10.30	23.92 =15.50 =17.03 -30	44.58 -34.96 -18.30 5.01	72.74 =36.56 =17.84 0.49	106.95 -33.85 -15.27 -4.93	143.62 -25.61 -10.60	177-15 *20-66 *5-77 *15-93
	41.00 3 PRESS. 6 PRESS. 7 PRESS.	0.15 -25.35 -5.91 8.75	7.82 -28.93 -7.64 8.65	9.87 -32.05 -9.33 7.85	22.52 -34.30 -10.70 4.16	41.64 *35.19 *11.37 3.50	67.25 -34.31 -10.96 -0.08	97.61 -31.32 -9.29 -4.32	130.01 -26.12 -6.56 -8.75	157.00 *16.63 -3.45 *12.72
HOR. THEF VER. DRAG VER. THEF VER. THEF VER. THEF	30.20 PRESS. PRESS. PRESS.	7.07 -25.42 -3.41 6.46	-2.42 -24.71 -4.35 4.33	9.00 -31.50 -5.24 5.67	20.85 -33.34 -5.95 4.36	3A.60 =33.81 =6.25 34	42.11 -32.58 -5.98 -0.33	69.61 -29.41 -5.03 -3.45	118.65 -24.27 -3.53 -6.67	144.31 -17.34 -1.85 -7.53
HOR. DEAL VER. DEAL VER. INC.	20,00 T PBF55. T PBF55. T PRE55.	0.01 -25.45 -2.12 4.26	2.09 -28.57 -2.15 4.15	7.94 -31.09 -7.14 3.68	18.98 -37.64 -7.63 -7.78	35.54 =37.83 =7.74 1.43	57.34 -31.36 -2.67 -0.75	77.59 -28.07 -2.14 -2.40	109.27 -22.98 -1.52 -4.51	132.58 -14.31 -0.79 -6.36
MOR. DRAG MOR. PAFF VER. DRAG VER. TAFF	In on PPFSS. PPFSS. PPFSS.	-0.94 -25.46 -0.50 2.12	1.75 -24.46 -0.65 -0.96	6.74 -10.88 -0.59 1.81	14.97 -32.21 -0.66 1.36	32.50 -32.23 -0.65 0.67	97,97 -10,43 -0,64 -0,21	76.95 -27.28 -0.54 -1.23	101.66 -22.22 -0.37 -2.78	123.42 -15.71 -0.19 -3.19
HOR THEN HOR THEN VEH GRAN VER THEN	0,00 • PHF55 • PHF55 • PHF55 • PHF55	-0.42 -25.85 -0.41 -0.00	0.08 -28.42 0.00 0.00	0.47 =10.75 0.00 0.00	1.17 -12.07 -0.00 -0.00	2:31 =37:04 ():00 0:00	7.813 #30.59 0.50 0.00	5.64 -27.01 0.00 0.00	7.50 -21.47 0.00	9.14 19.91 0.00

WAVE: CREST: 93 FT. DEPTH

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	SURFACE = ELEVATION (FT.)	141.55	147,02	141,55	132,55	174,27	117.14	111,10	106.04	101,86
•	SURFACE MOR. DRAG PRESS. HOR. FIRN PRESS. VER. DRAG PRESS. HOR. DRAG PRESS. HOR. THE PRESS. VER. DRAG PRESS. VER. DRAG PRESS. VER. THE PRESS.	239.57 -47.09 -76.81 -49.73 490.16 -68.30 -70.81 -50.89	426.34 0.00 0.00 -67.90 714.31 0.00 -70.07	739.57 67.09 76.41 -49.73 490.16 64.30 70.81	16R.01 87.74 150.91 720.99 16R.01 82.34 150.91	157.46 77.91 171.74 -1.37 0.00 0.00	213.36 70.70 162.96 11.37 0.00 0.00 0.00	40.50 134.77 20.82 0.00 0.00 0.00	51.36 51.63 111.63 27.09 0.00 0.00	23.26 41.57 29.49 0.00 0.00 0.00
	HOR. DRAG PRESS. HOR. DRAG PRESS. VER. DRAG PRESS. VER. INCR PRESS.	775.06 -49.40 -44.37 -53.77	975.10 0.00 0.00 -67.35	775.06 49.40 44.37 -53.77	453.49 78.14 136.11 -23.40	155.46 77.01 171.74 -1.37	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00
	HOR. DRAG PRESS. HOR. THER PRESS. VER. DRAG PRESS. VER. INER PRESS.	843.78 *36.63 *28.69 *51.97	845.19 0.00 0.00 -61.72	#n3.78 38.63 28.69 -51.97	639.45 63.86 91.68 -29.04	425.35 73.18 148.29 -5.01	215.36 70.70 162.96 11.37	40.50 62.83 139.77 20.62	0.00 0.00 8.00 0.00	0.00 0.00 0.00 0.00
	FLEV = 110.00 HOR. DRAG PRESS. HOR. INER PRESS. VER. DRAG PRESS. VER. INER PRESS.	536.10 -30.75 -16.97 -48.07	575.90 0.00 0.00 -55.18	536.10 30.75 18.97 -48.07	432.21 52.61 62.52 -30.63	303.28 63.01 105.12 -10.76	188.94 65.09 130.53 6.39	104.93 62.17 135.43 20.02	51.36 53.64 111.63 27.09	23.26 43.97 84.57 29.49
	FLEY = 100.00 HOR. DRAG PRESS. HOR. INER PRESS. VER. DRAG PRESS. VER. INER PRESS.	438.39 -24.90 -12.73 -43.30	469.08 0.00 0.00 -48.59	438.39 24.90 12.73 -43.30	43.75 42.94 -29.93	262-65 54-27 74-40 -13-71	171.98 57.00 95.07 1.33	100.60 56.44 101.04 13.72	\$2.15 51.35 95.10 22.94	22.90 43.67 60.80 28.25
	PELEY # 90 00 HOR. ORAG PRESS. HOR. INER PRESS. VER. INER PRESS.	366.05 =20.49 =8.60 =38.29	188.77 0.00 0.00 -42.27	346.05 20.49 6.60 -38.29	304.46 34.78 29.54 -27.96	230.41 46.91 52.41 -14.82	156.16 51.32 68.58 -1.96	95.71 51.27 74.91 9.00	\$2.34 47.80 72.12 17.32	24.63 61.99 62.72 22.86
	PRESS. AO PO	309.46 *17.13 *33.13	324.76 0.00 7.00 -36.36	309.46 17.13 -33.33	267.49 31.27 20.23 -25.30	202.71 40.82 34.58 -14.71	141.58 45.73 48.88 -3.92	90.21 46.70 54.70 5.57	\$1.40 44.56 53.69 12.95	25.45 40.23 47.66 17.74
	HOR. DRAG PRESS. HOR. INC. PRESS. VER. INC. PRESS. VER. INC. PRESS.	264.82 =18.54 =3.89 =28.55	278.34 0.00 0.00 -30.87	264.82 14.54 3.89 -28.55	778.40 76.93 13.69 -77.30	179.11 35.64 25.11 -13.62	128.83 47.99 34.32 -4.90	54.38 47.73 39.06 3.15	49.64 41.66 39.00 9.58	25.51 38.52 35.24 13.97
	MOR. DRAG PRESS. MOR. INER PPESS. VER. DRAG PPESS. VER. INER PRESS.	279.38 -12.56 -2.55 -23.99	240.23 0.00 0.00 -25.77	779.38 17.56 2.55 -73.99	199.85 23.52 9.05 -19.14	159.79 31.81 16.67 -12.42	117.28 37.05 23.46 =5.18	78.49 39.36 27.07 1.57	47.30 39.12 27.38 4.99	34.94 36.92 25.12 10.86
	MOR. DRAG PRESS. HOR. TORAG PRESS. VER. TORAG PRESS. VER. TNET PRESS.	201.16 -11.03 -1.61 -19.45	710.11 0.00 0.00 -21.01	901.18 11.03 1.61 -19.65	177.15 20.86 5.77 -15.93	143.62 29.41 10.88 -10.69	106.95 33.05 15.27 -4.93	72.74 36.56 17.84 0.49	44.58 36.96 18.30 5.01	73.92 35.50 17.03 0,30
	MOR, DRAG PRESS. MOR. JREA PRESS. VER. DRAG PRESS. VER. INFR PRESS.	179.35 =9.88 =9.94 =15.51	186.69 0.00 0.00 16.51	179.35 9.88 0.96 -15.51	159.00 18.83 3.85 -12.72	130 • 01 26 • 12 4 • 46 -4 • 75	97.81 1.32 .29 .37	67.25 34.31 10.96 -0.08	41.64 35.19 11.37 3.50	22.52 34.36 10.70
	MOR. DRAG POFSS. MOR. DRAG POFSS. WOR, JNEW POFSS. VER. DRAG POFSS. VER. INF POFSS.	167.17 -9.04 -0.51 -11.52	168.59 0.00 9.00 -12.21	167.17 9.04 0.51 -11.52	144.31 17.34 1.85 -9.53	119.65 24.27 3.53 -4.47	49.81 22.41 5.03 -3.45	72 · 11 32 · 58 5 · 98 -0 · 33	36.60 33.61 6.75 7.34	20.85 33.34 3.95 4.36
	MOR. OKA: PRESS. MOR. INEM PRESS. VER. INER PRESS. VER. INER PRESS.	taA.79 =8.47 =9.22 =7.44	154.50 0.00 9.00 -4.10	148.72 8.47 9.27 -7.64	132.58 16.31 0.79 =4.36	109.57 22.28 1.52 -4.51	#2.69 75.07 2.18 -2.49	57.34 31.36 -0.35	35.54 37.63 2.74 1.43	19.96 37.64 7.63 2.78
	MOR. DRAG PRESS. MOR. DRAG PRESS. VER. DRAG PRESS. VER. THE PRESS.	139.45 -8.14 -0.05 -1.82	143.82 0.00 1.00 14.04	13 4.45 7.14 7.75 -3.82	123.42 15.71 -1:17	101.66 27.22 0.17 -2.28	76.95 27.26 0.54 *1.23	52.97 10.63 0.64 -0.21	32.50 32.23 0.67 0.67	16.97 32.21 0.86 1.76
	PLEY # 0,000 HOL, CHA; PUTTO HOR, INEM POTTO VEW CHAS PUTTO VEW THE POTTS	10.27 -8.03 6.06 0.00	10.48 9.00 0.75 9.00	10.27 9.03 0.00 0.00	7.14 19.51 0.70 0.00	7.50 71.97 0.60 0.00	5 . 44 27 - 01 0 - 00 0 - 00	7.84 30.37 0.00 0.00	7.31 37.04 0.00	1.17 37.07 0.00

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**** x = (FT.)	140.00	180.00	200,00	270,00	240.00	760.00	280.00	300.00	320.00
SURFACE = ELEVATION (FT.)	98,4R	95,72	93,50	91.73	90.37	89.22	88.35	87,68	87.17
NUMPACE PRESS. MON. DRAG PRESS. VEN. DRAG PRESS. VEN. DRAG PRESS. MON. DRAG PRESS. MON. DRAG PRESS. VEN. DRAG PRESS. VEN. DRAG PRESS. VEN. DRAG PRESS.	8.05 35.42 61.39 25.72 0.00 0.00 0.00	1.78 28.27 26.47 0.00 0.00 0.00	0.02 23.71 23.91 0.00 0.00	-0.67 12.01 12.15 21.15 0.00 0.00	-2.40 13.69 17.96 0.00 0.00 0.00	-4.43 10.51 8.05 14.70 0.00 0.00 0.00	-6.43 8.37 5.01 12.04 0.00 0.00	-8.27 4.59 3.02 10.22 0.00 0.00 0.00	4.77 4.77 1.72 0.00 0.00 0.00
HOR. DRAG PRESS. HOR. INEH PRESS. VER. DRAG PRESS. VER. INER PRESS.	0.00 0.00 0.00 2.00 2.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00
HOR. DRAG PRESS. HOR. THER PRESS. VER. THER PRESS. VER. THER PRESS.	2.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	7.70 0.01 0.00 0.00	7.00 0.00 0.00 0.30	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00
ELEY # 110.00 HOR. ORAG PRESS. HOR. INER PRESS. VER. DRES PRESS. VER. INER PRESS.	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00
HOR. DRAG PRESS. HOR. INER PRESS. VER. THER PRESS. VER. THER PRESS.	8.05 35.42 61.39 28.72	1.78 28.62 43.27 26.47	0.02 23.03 29.71 23.91	-n.67 10.01 19.76 21.15	-2.40 13.69 12.75 17.96	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00
FLEY # 90.00 HOR. DRAG PRESS. HOR. INER PRESS. VER. DRAG PRESS. VER. INER PPESS.	9.25 15.10 50.45 24.84	7.15 28.97 36.24 24.16	0.04 23.35 27.69 22.74	*0.62 19.22 19.13 20.65	-2.39 13.74 12.67 17.89	-4.43 10.51 8.05 14.70	-6.83 9.37 5.01 12.04	-8.27 6.59 3.02 10.22	-9.79 4.77 1.72 8.76
FLEY # AC 00 HOR. DRAG POESS. HOR. THER POESS. VER. DRAG POESS. VER. THER POESS.	10.22 34.83 39.12 20.07	2.70 29.31 30.25 20.40	7.13 24.11 22.23 19.53	-0,45 19,31 15,58 17,91	+2.18 15.09 10.47 15.73	-4.39 11.72 6.61 13.30	-6.60 9.21 4.30 11.14	-8.60 7.15 2.63 9.45	-10.26 5.26 1.51 6.12
HORLEY APPESS. HOR. THEY PRESS. VER. ORAG PRESS. VER. INEN PPESS.	10.73 34.17 29.46 16.32	3.04 29.39 23.16 17.00	0.21 24.65 17.26 16.54	-0.37 20.17 12.26 15.34	-7.12 16.15 6.33 13.63	-4.49 12.79 5.45 11.71	*6.93 10.09 3.46 9.93	-9.16 7.65 2.11 8.46	-11.04 5.66 1.23 7.22
HOR. DRAG PRESS. HOR. INER PRESS. VER. DRAG PRESS. VER. INEH PRESS.	10.90 33.43 21.35 13.09	3.17 29.31 17.03 13.94	9.24 25.01 12.86 13.77	-0.36 20.82 9.23 12.90	*2.19 16.97 6.32 11.59	-4.74 13.64 4.16 10.08	-7.41 10.64 2.64 8.62	-9.88 8.47 1.62 7.37	-11.98 6.47 0.94 6.29
MORLEY = 50.00 MOR. DRAG PRÉSS. VER. DRAG PRESS. VER. INFO POESS.	10.49 32.71 14.47 10.30	3.09 20.14 11.86	0.21 25.23 9.07 11.18	-0.41 21.31 6.56 10.58	-2.40 17.61 4.51 0.50	=5.17 14.31 2.97 6.42	-8.07 11.46 1.89 7.26	-10.78 7.00 1.16 6.22	-13.10 6.88 0.65 5.32
ELEY # 40.00 MOR. DRAG POESS. MOR. INER PRESS. VER. INER PRESS. VER. INER PRESS.	9.87 32.05 9.13 7.85	2,42 24.93 7.64 8,65	0 • 15 25 • 35 5 • 91 8 • 75	-0.57 21.66 4.47 8.35	•2.77 14.10 2.95 7.63	-5.78 14.84 1.95 6.75	-8.94 11.95 1.24 5.86	-11.89 9.44 0.76 5.03	-14.43 7.26 0.45 4.32
HORE TO BOOK TO THE TOTAL TO THE TOTAL TOT	9.03 11.50 5.24 5.47	2.42 24.73 4.35 6.33	0.07 25.02 3.81 4.46	-1.49 21.97 2.81 4.21	-1.42 14.45 1.74 5.71	*6.61 15.23 1.11 5.07	=10.03 12.33 0.71 4.42	=13.73 9.75 0.44 3.81	-16.00 7.54 0.26 3.26
HOR INFO POTES	7.94 31.09 2.34 3.48	2.09 28.57 2.15 4.15	7.01 25.45 2.07 4.26	-1.44 22.06 1.30 4.11	-4.23 18.69 2.72 1.80	-7.47 15.50 0.50 3.39	-11.3A 12.59 0.32 2.95	-14.43 10.02 0.20 2.54	-17.82 7.75 9.20
FLEV = in on MRR. DRAG PUTS. WIR. T.I. POTS. VEW JUTS. VEW JUTS.	6.74 30.84 0.54 1.81	1.75 24.44 0.65 2.04	-0.04 25.46 0.50 2.12	-7.71 77.15 0.36	-1.16 18.87 1.17 1.70	15.65 15.65 0.75	-13.02 12.75 0.09	-16.73 10.16 0.05	-10.03 7.07 0.03 1.11
FLEV & Agno MON. 1-AC HOST MON. 1467 POINTS VEN. 1467 POINTS VEN. (ALV. PARTY VEN. (ALV. PARTY)	0,43 30.75 7,07 0,10	0,08 28,42 0,05 0,90	=0.02 25.46 6.01 0.03	-1.24 22.14 1.11 0.11	#0.50 18.87 11.60 0.00	-0.2 15.71 0.20 0.20	-1.18 12.80 0.00 0.00	*1.49 10.71 0.60 0.60	*1.75 7.00 0.00

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MOR. DRAG PRESS. MOR. INEW PRESS. VER. THER PRESS. VER. INER PRESS.	0.00 0.00 0.00 0.00	0.00 0.30 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	*************************************
PLEY 120.00 MOR. DRAG PRESS. MOR. INER PRESS. VER. DRAG PRESS. VER. INER PRESS.	0.00 0.00 0.00 0.00	n.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	
HOR. DRAG PRESS. HOR. THER PRESS. VER. TRAG PRESS. VER. THE PRESS.	0.00 0.70 0.70 0.70	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	***************************************
FLEY # 100.00 HOR. DRAG PRESS. HOR. INER PRESS. VER. DRAG PRESS. VER. INTR PRESS.	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	***************************************
ELEV = 90.00 MOR. DRAG PRESS. MOR. INER PRESS. VER. TRAG PRESS. VER. INER PRESS.	-10.91 3.18 0.90 7.26	711.70 2.20 0.43 5.96	-12.27 1.59 0.17 5.33	-12.64 0.79 0.03 5.29	
ELEV # 90.00 HOR. DRAG PPFSS. HOR. INER PRFSS. VER. INER PRESS.	-11.51 -1.64 -0.31 -6.81	~12.40 2.51 7.39 5.71	-13.02 1.69 0.15 5.12	-13.40 0.75 0.03 5.00	
ELEY = 70 nn Hor. Drag Press. Hor. Inem Press. Ver. Drag Press. Ver. Inem Press.	*12.50 4.21 0.66 4.12	*13.57 2.91 0.32 5.22	-14.31 1.85 0.17 4.69	-14.74 C.81 0.02 4.50	
HOR. DRAG PRESS. HOR. THER PRESS. VER. THER PRESS. VER. THER PRESS.	*13.65 4.48 7.51 5.36	-14.89 3.24 0.25 4.63	-15.75 2.02 0.09 4.17	-16.22 0.65 0.02 1.97	
HORLEY B 50.00 HORLE DRAG PRESS. HORLE DRAG PRESS. VER. INFR PRESS.	-14.07 5.05 0.37 4.55	-16.38 3.52 0.18 3.97	-17.34 2.17 0.07 3.56	-17.67 0.91 0.01 3.39	,
FLEY 40.00 HDR. DRAG PPESS, HDR. 114FR PPESS, VER. DRAG PPESS, VER. INCR PRESS.	-14.50 5.37 0.24 3.71	*18.06 3.74 0.12 3.24	-19.13 2.30 0.04 2.93	-17.71 0.95 0.01 2.77	
FLEY # 30.00 HOR. DRAG PRESS. HOR. INFO PRESS. VER. DRAG PRESS. VER. INFO PRESS.	-18.25 5.40 0.14 2.42	=19.96 3.91 0.07 2.07	-21.13 2.40 0.03 2.24	-21.77 0.92 0.00 2.11	
FLEV = 20,00 HOR ORAG PERSS HOR INER PRESS VER ORAG PRESS	-20.25 5.77 2.06 1.40	-72.10 4.03 0.03 1.67	-23.16 2.47 7.01 1.51	-24,06 1.02 0.00 1.43	
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APPENDIX B.1

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U.S. NAVY - ACMR PLATFURMS - PLATFURM 2 .	G. BUCHMASTEK	JULY, 1976	UNITS	UDIPUT GAITS					

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587.0	103	1034.4	1184.8	-68695.	50505	102239.	-114.2
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500,1	80	886.0	1016.9	-10201-	42510,	855219	-104,2
7.797	9	0.508	921.0	-66247.	38328,	76556.	1.66-
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REFURTERS.	639,1422 KIPS	1162,0762 KIPS 1326,2443 KIPS	=99252.6111 FT=KIP3	53831.7455 FT-KIP3	112911,1935 FT-KIPS	-100,0843 KIP3					
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| ### FINDENTANCE VERTICAL PURCE RIPS NT Z |
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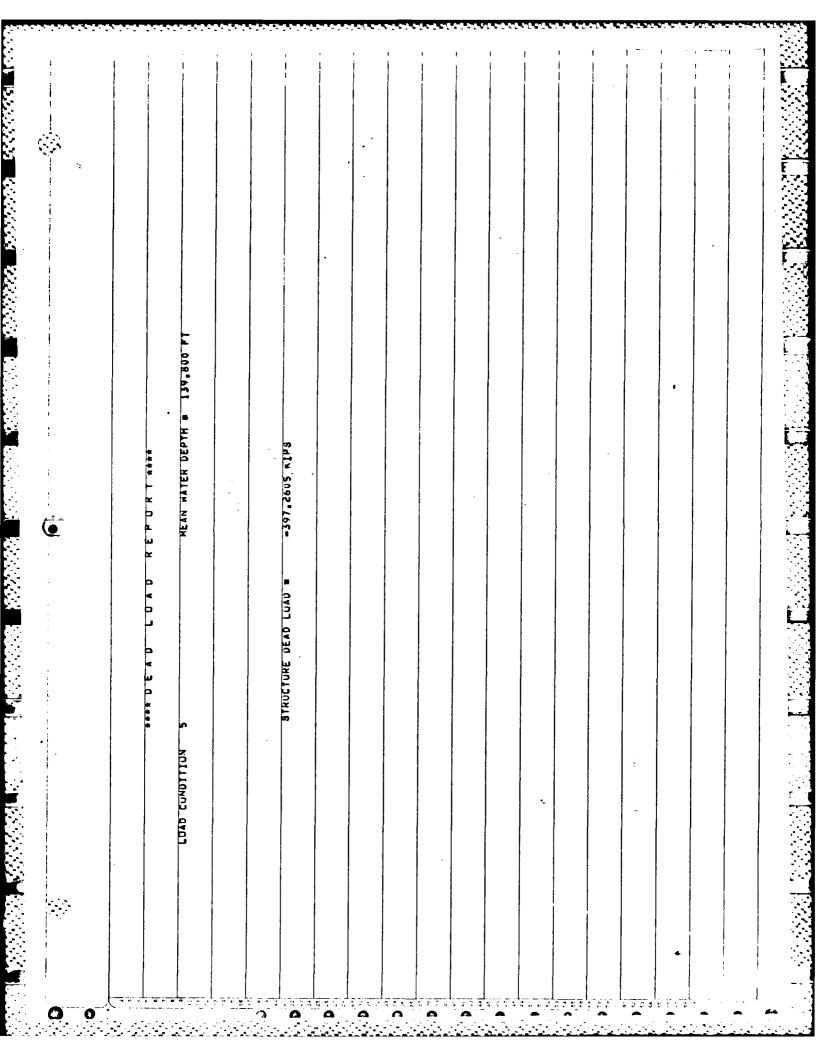
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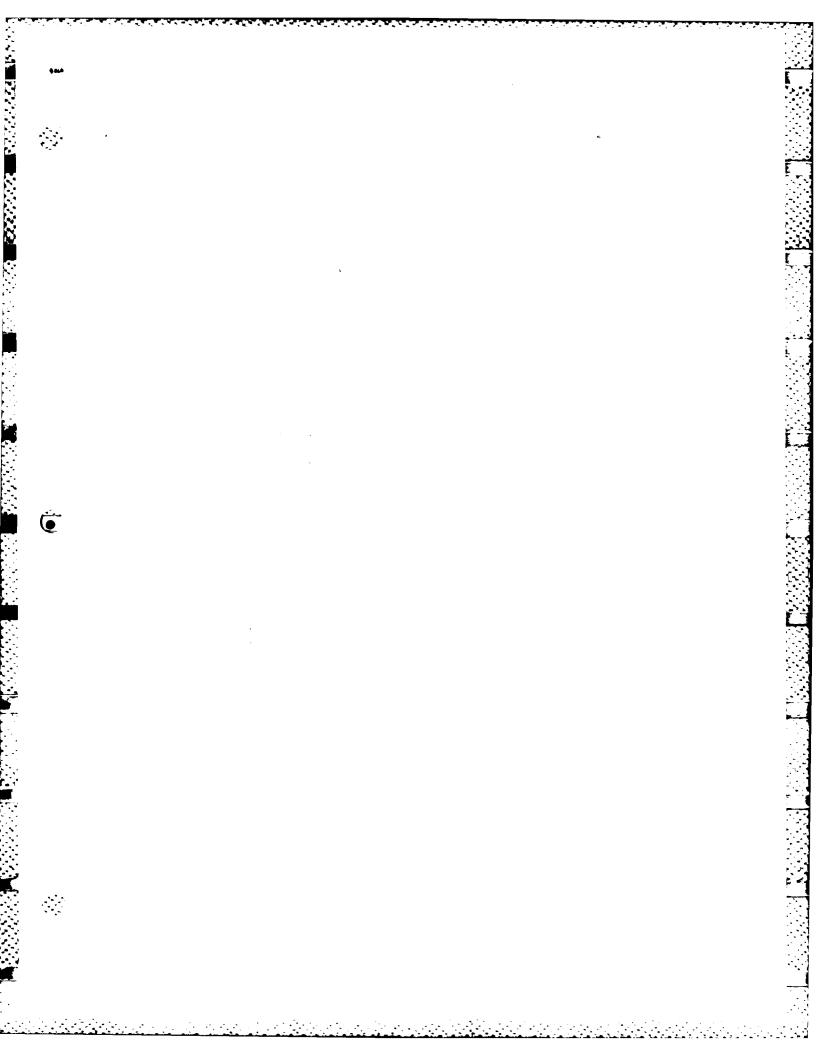


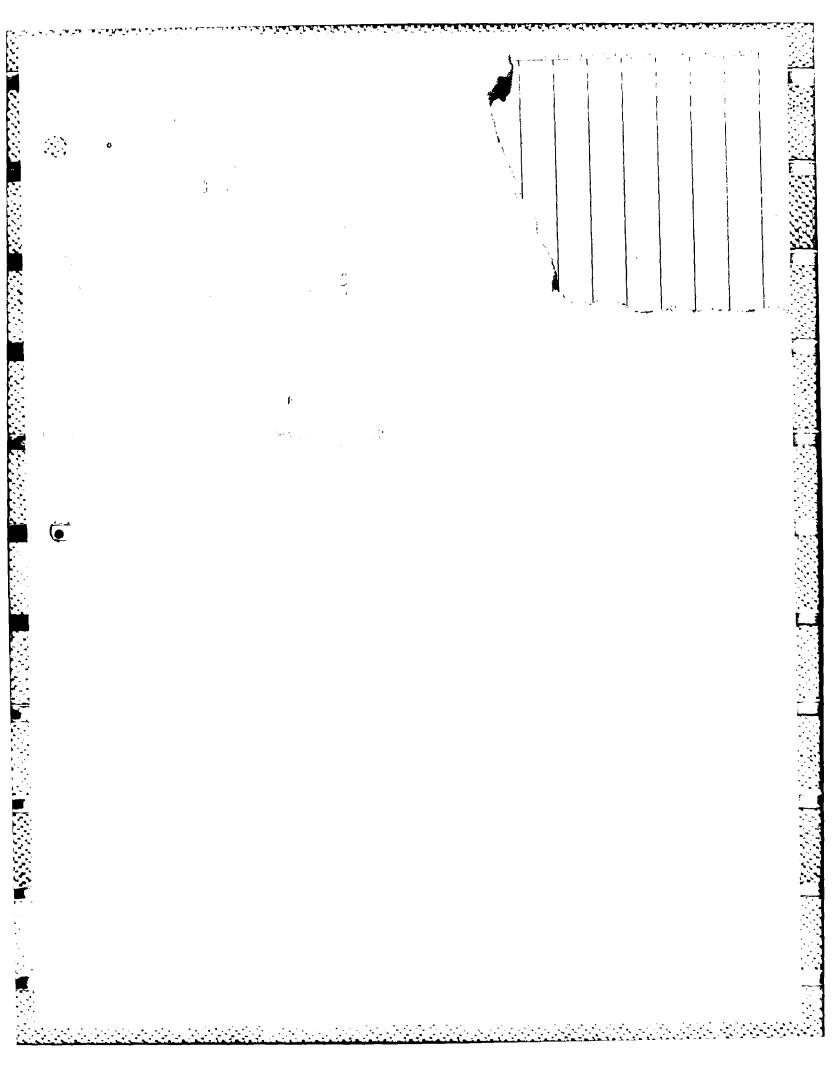
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(FI) = 14.00 (EXIL) 0.000 .356 .611 .260 1.136 1.440 2.274 (EXIL) 0.000 1.204 1.381 2.347 2.896 3.644 5.830 (IX) 0.000 1.204 1.381 2.347 2.896 3.644 5.830 (IX) 0.000 1.354 1.834 2.494 3.384 4.594 4.594 (IX) 0.000 1.354 1.834 2.494 3.384 4.594 4.594 (IX) 0.000 1.354 1.834 2.494 3.384 4.594 4.594 (IX) 0.000 1.354 1.834 2.494 3.384 4.599 2.000 (FI) = 45.00 (FI) = 45.00 (IX) 0.000 2.321 3.150 4.275 5.802 7.875 7.875 (IX) 0.000 2.324 3.43 4.719 5.791 7.259 11.615 (IX) 0.000 3.542 4.840 6.880 6.880 7.00 1.580 (IX) 0.000 3.542 4.880 6.880 8.210 10.303 15.485 (IX) 0.000 3.542 4.880 6.880 8.210 10.303 15.485 (IX) 0.000 4.75 6.551 9.079 11.176 14.040 22.464 (IX) 0.000 4.75 6.551 9.079 11.176 14.040 22.464 (IX) 0.000 4.75 6.551 9.280 4.40 3.00 1.580	i	000.0	570	120	0.75.	.430	.700	1,580	20,000	
(FI) = 24.00 (FI) = 24.00 (IV) 0.000 1.204 1.281 2.347 2.896 3.644 5.830 (IV) 0.000 1.204 1.281 2.347 2.896 3.644 5.830 (IV) 0.000 2.799 3.444 4.456 5.337 6.572 10.515 (IV) 0.000 1.354 1.834 2.494 3.384 4.594 4.594 (IV) 0.000 1.354 1.834 2.494 3.384 4.594 4.594 (IV) 0.000 2.321 3.150 4.275 5.802 7.875 7.875 (IV) 0.000 2.321 3.150 4.275 5.802 7.875 7.875 (IV) 0.000 2.532 3.433 4.719 5.791 7.659 11.615 (IV) 0.000 3.542 4.840 6.890 4.400 1.500 1.580 (IV) 0.000 3.542 4.840 6.890 3.440 7.00 1.580 (IV) 0.000 3.542 4.840 6.890 3.440 7.00 1.580 (IV) 0.000 3.542 4.840 6.890 3.440 7.00 1.580 (IV) 0.000 3.542 4.840 6.890 3.440 7.00 1.580 (IV) 0.000 3.542 4.840 6.890 3.440 7.00 1.580 (IV) 0.000 3.542 4.840 6.890 3.440 7.00 1.580 (IV) 0.000 3.542 4.840 6.890 3.440 7.00 1.580	3	14.00	771	117	100				17 m c c	
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	[2] [3]	6000	.036	110	200	0277	707	1.580	20.000	
(FI) = 33,00 (A/IN) 0,000 2,749 3,444 4,456 5,337 6,572 10,515 (A/IN) 0,000 1,354 1,834 2,494 3,384 4,594 4,594 (IN) 0,000 2,321 3,150 4,275 5,802 7,875 7,875 (A/IN) 0,000 2,524 3,433 4,719 5,791 7,259 11,615 (A/IN) 0,000 3,542 4,840 6,840 8,210 10,503 16,485 (IN) 0,000 3,542 4,840 6,840 8,210 10,503 16,485 (IN) = 90,00 (IN) = 90,00 (IN) = 90,00 (IN) = 90,00 (IN) = 90,00 (IN) = 90,00		000000	1.204	1.581	2.347	2,846	3.644	5,830	5,630	
(FI) = 33.08 (N/IN) 0.000 1.354 1.834 2.494 3.384 4.594 4.594 (N/IN) 0.000 1.354 1.834 2.494 3.384 4.594 4.594 (N/IN) 0.000 2.321 3.150 4.275 5.802 7.875 7.875 (N/IN) 0.000 2.321 3.150 4.275 5.802 7.875 7.875 (N/IN) 0.000 2.321 3.150 4.275 5.802 7.875 7.875 (N/IN) 0.000 2.321 3.433 4.719 5.791 7.259 11.615 (N/IN) 0.000 3.542 4.840 6.880 8.210 10.503 16.485 (N/IN) 0.000 3.542 4.840 6.880 8.210 10.503 16.485 (N/IN) 0.000 4.756 6.551 9.079 11.176 14.040 22.464 (N/IN) 0.000 4.756 6.551 9.079 11.176 14.040 22.464 (N/IN) 0.000 4.756 6.551 9.079 11.176 14.040 22.464 (N/IN) 0.000 4.756 6.551 9.079 11.176 14.040 25.464 (N/IN) 0.000 4.756 6.551 9.079 11.176 14.040 25.464	(F. C. X.)	33.00	2	9 9 9	4 3	211	3	2		
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$		0000	120	180	310	450	700	1,580	20,000	
(11) 0.000 1.554 1.654 2.494 3.384 4.594 4.594 (11) 0.000 0.064 1.654 0.400 1.010 2.520 20.000 (11) 0.000 2.321 3.150 4.275 5.802 7.675 7.875 (12) 0.000 2.000 2.52 3.43 4.719 5.79 7.29 11.619 (11) 0.000		33.08								
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$		0000	1.554	1.854	4.0	3.384	2.50 2.50 5.00 5.00	2000		
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	PTH (FF) B HCE (K/IN) FL (12)	00.000	2.321	3,150	4.275	5.802	7.675	7,875		
(1.1) = 65.00 , 0.74 , 150 , 290 , 440 , 700 1,580 (1.1) = 65.00 , 0.00 3,542 4,840 6,840 8,210 10,503 16,485 (1.1) 0,000 4,756 6,551 9,079 11,176 14,040 22,464 (1.1) 0,000 4,75 6,551 9,079 11,176 14,040 22,464 (1.1) 0,000 4,75 6,551 9,079 11,176 14,040 1,580 (1.1) = 90.00	214 (*1.) # 208 (*/12.)	65°00	2,532	3,453	4.719	5.791	7.259	11.615	11.015	
(F.1) = 65.00 (F.12.) 0.000 3.542 4.840 6.840 8.210 10.503 10.485 (I.1) 0.000 .075 .150 .280 .440 .700 1.580 (F.1) = 90.00 (F.1) = 90.00 4.756 6.551 9.079 11.176 14.040 22,464 (F.1) = 90.00 .073 .140 .280 .440 .700 1.580	3	0.000	670.	.150	062.	044	007.	1,580	20,000	
(*/IN) = 90.00 4.756 6.551 9.079 11.176 14.040 22,464 (1.1) 0.000 .075 .075 11.176 14.040 1.580 (1.1) = 90.08		000000000000000000000000000000000000000	3,542	4.840	6.68U	8.210	10.503	16,485	16,485 20,000	
(F1) H 20.0B	(FI) # (A/In) (A/In) (I)	, 3	4.756	6,551	9.079	11.176	14,040	1.580	22,464 20,000	
(*/1.4) 0.000 1.494 3,003 4.684 5,988 7.706 1	(F1) (A/14	00000	763.1	3,003		•	7.706	12,329	12,329	

2 08/27/76 PAGE U.S. NAVY - ACHR PLAIFURMS - PLAIFURM NU. 2 - MML 93.0 FEET - 50 YR STURM 13,001 15,350 15.350 13,001 15,350 15,350 STRAN - Pev DATA 4.126 9.594 7.007 7.452 6.314 7.452 54833 4.945 5.433 3.728 3.728 3.167 12636 1.638 1.541 114.00 0.000 0.000 PILE JUINT NO. 1010 DEFL (10) B UEPTH (FT) # PINGE (K/IN) UEFL (IN)

THE PERSON NAMED IN COLUMN TO PERSON NAMED I

.000 .000	150 1,440 150 1,044 120 1,00 120 1,00 120 1,00 120 1,00 120 1,00	237 1,580 20,000 2,274 2,274 1,580 20,000 1,585 24,000 1,585 24,000 1,586 20,000
(FI) = 5.00 (FI) = 0.000 0.000 (FI) = 0.000 0.000 (FI) = 24.00 (FI) = 24.00 (FI) = 24.00 (FI) = 24.00 (FI) = 33.00 (FI) = 6.000 1.354 1.436 2.494 (FI) = 45.00 (FI) = 45.00 (FI) = 45.00	3.044 1.444 1.444 1.00 2.572 3.004	560 560 560 560 560 560 560
(FI) = 5,00 (FI) = 9,00 (FI) = 9,00 (FI) = 0,000 (FI) = 19,00 (FI) = 24,00 (FI) = 24,00 (FI) = 24,00 (FI) = 24,00 (FI) = 33,00 (FI) = 45,00 (FI)	3.044 1.440 1.440 1.00 2.572 1.00	560 560 560 560 560 560 560
(*/! = 9.00 (*/! ×) = 0.000 (*/! ×) = 0.000 (*/! ×) = 10.000 (*/! ×) = 24.00 (*/! ×) = 24.00 (*/! ×) = 24.00 (*/! ×) = 33.00 (*/! ×) =	3.044 3.044 3.044 5.00 7.00 7.00	56.0 56.0 56.0 56.0 56.0 56.0 56.0
(F) = 24.90 (F) = 24.90 (F) = 24.90 (F) = 33.00 (F) =	3.044 1.444 3.044 3.044 7.00 7.00	00 00 00 00 00 00 00 00 00 00 00 00 00
(FI) = 144,000	3.0440 3.044 3.044 5.000 5.572 1.00	56.5 56.5 56.5 56.5 56.5 56.5 56.5 56.5
(FI) = 24,90 (IV) = 33,00 (IV) = 33,00 (IV) = 33,00 (IV) = 33,00 (IV) = 33,00 (IV) = 55,00 (IV) = 55,00 (IV) = 55,00 (IV) = 55,00 (IV) = 55,00 (IV) = 6,000 (IV) = 4,00 (IV) =	3.044 200 200 200 3.000	586
(FT) = 33.00 (A/IN) 9.000 (1N) 0.000 (1N) 0.000 (FT) = 55.00 (K/IN) 0.000 (A/IN)	573 007.	515
(FI) = 55.00 (K/I) 0.000 1.554 1.436 2.494 (L.) 0.000 .054 1.656 .400 (FI) = 45.00 .050 2.321 3.150 4.275 (K/IN) 0.000 2.321 3.150 4.275 (FI) = 45.00		9
(F1) = 45.00 (N/IN) 0.000 2.321 3.150 4.275 (IV) 0.000 .964 .160 .400 (F1) = 45.05	3.384 4.594 4. 1.010 2.520 20	4,594 20,000
0.60° 22° 23° 23° 23° 23° 23° 23° 23° 23° 23	5.802 7.875 7. 1.010 2.520 20	7,875 22,200
0.000 0.000 0.000 (C.1.)	791 7 259 440 . 700	11.615 11.615 1.580 20.000
UEPTH (F.) = 65,00 6.000 5.542 4.440 6.680 8. UEPL (14) 0.000 .075 .150 .240 .	8.210 10.305 16.	6,485 16,485 1,580 20,000
UEPIN (FI) H 90.00 4.756 6.551 9.079 11. UEFL (1.7) 0.000 4.756 1140 .279 11.	11.176 14.040 22.	22,464 22,464 1,580 20,000
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	5,988 7,706 12,	928.621 928.63

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08/27/76 PAGE - ACMR PLATFURMS - PLATFURM NO. 2 - HML 93.0 FEET - 50 YR STURM 13.001 15,350 15.350 15.350 13,001 15,350 A N = P=Y D A T 9.594 6.126 6.314 7.452 7.452 4.945 5.833 5.835 3,167 3.728 3,728 U.S. RAVY 1.638 1.830 114,00 0,000 0,000 00000 PILE JUINT NU. 1011 DEPTH (FT) #
FINCE (N/IN)
DEFL (IN) VENTA (FI) B FUNCE (N/IN)

$ \begin{array}{cccccccccccccccccccccccccccccccccccc$			U.S. NAVY	IY - ACHR PLATE	PLATEURN	S PLAIF	URM_NO2	E6-188	UKNSPLAIFURM NO2MAL-93.0 FEET50 YK STORM	DATE	06/27/76
(11) 00000 0,000 (11) = 5,000 (11) 0,000 0,000 (11) 0,000 0,000 (11) 0,000 0,000 (11) 0,000 0,000 (11) 0,000 0,000 1,200 1,120 1,130 1,440 2,274 (11) 0,000 0,000 1,200 1,200 1,200 1,130 1,420 2,200 (11) 0,000 1,200 1,200 1,200 1,200 1,200 1,200 (11) 0,000 1,200 1,200 1,200 1,001 2,407 2,400 3,204 2,500 (11) 0,000 1,200 1,200 1,001 2,404 3,384 4,594 1,500 (11) 0,000 1,000 1,000 1,000 1,000 1,000 1,010 2,520 20,000 (11) 0,000 1,000 1,000 1,000 1,000 1,000 1,010 2,520 20,000 (11) 0,000 1,000 1,000 1,000 1,000 1,000 1,000 1,500 (11) 0,000 1,000 1,000 1,000 1,000 1,000 1,000 1,500 (11) 0,000 1,0	(FT) E	•									
((17) = 9.00 ((17) = 9.00 ((17) = 9.00 ((17) = 9.00 ((17) = 0.000 ((17) = 0.000 ((18) = 0.	FUNCE (KIIN) DEFL (IN)	0.00.0	000.05								
	(+1) R	5.00									
(*\i\) = \(\frac{4.00}{4.00}\)	E (K/IN)	0.00.0	20.000								
(*\I.*)		9.00									
(FI) = 14,00 (FI) = 14,00 (FI) = 24,00 (FI) = 24,00 (FI) = 24,00 (FI) = 35,00 (FI) = 45,00 (FI) = 40,00 (FI)	0.000	101.	12	155	169	186	.237	.237			
(+1) = 14,00 (14) 0,000 0,030 .110 .260 1,130 1,440 2,274 (14) 0,000 1,204 1,081 2,347 2,896 3,044 5,830 (14) 0,000 1,204 1,081 2,347 2,896 3,044 5,830 (14) 0,000 2,749 3,444 4,850 7,70 1,580 (14) 0,000 2,120 1,834 2,494 3,884 4,594 4,590 (14) 0,000 2,321 3,130 4,275 5,802 7,875 7,875 (1,015) 2,000 (14) 0,000 2,321 3,130 4,275 5,802 7,875 7,875 (1,015) 2,000 (14) 0,000 2,321 3,130 4,275 5,802 7,875 7,875 (1,015) 2,000 (14) 0,000 4,750 4,890 3,890 3,440 1,000 22,464 (1,015) 10,303 16,485 (1,015) 1,000 1,200 1,260 (1,015) 1,000 1,200 1,260 (1,015) 1,000 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,200 1,2,200 1,200 1,2,2	?:	600°0	5 7 0 °	2	. 27 U	• 430	.700	•	20,000		
(*/1*) 0,000	(3)	3	į								
(11) = 24.00	33	000	.366	. 511	905	1,136	1,440	2,274	2.274		
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$		•	000	011.	207.		000	1.560	000.00		
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$		24.00									
(FI) = 33.00 (A/IN)		00000	.20	1.661	2.347	2.896	3.044	5,830	5,630		
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$		000.0	¢0.	140	280	430	707	1,560	20,000		
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	1	14.00									
(FI) = 33.08 (A/IN) 0.000 1.354 1.838 2.494 3.384 4.594 4.590 (A/IN) 0.000 1.354 1.838 2.494 3.384 4.594 4.594 (A/IN) 0.000 2.321 3.150 4.275 5.802 7.875 7.875 (A/IN) 0.000 2.321 3.150 4.275 5.802 7.875 7.875 (A/IN) 0.000 2.321 3.150 4.719 5.791 7.259 11.515 (A/IN) 0.000 4.750 4.180 5.840 8.210 10.303 16.445 (A/IN) 0.000 4.750 6.551 9.079 11.176 14.040 22.464 (A/IN) 0.000 4.750 6.551 9.079 11.176 14.040 22.464 (A/IN) 0.000 1.409 3.009 5.998 5.988 7.700 12.329	(~1/4)	000	2.789	7	450.0	5.337	575.0	10.515	20.01		
(+1) = 33.08 (-1/1) 0.000 1.354 1.838 2.494 3.384 4.594 4.594 (-1/1) 0.000 .000 .004 .100 4.275 5.802 7.875 7.875 (-1/1) 0.000 2.321 3.150 4.275 5.802 7.875 7.875 (-1/1) 0.000 2.321 3.433 4.719 5.791 7.259 11.615 (-1/1) 0.000 2.532 3.433 4.719 5.791 7.259 11.615 (-1/1) 0.000 3.542 4.840 0.290 .440 .700 1.580 (-1/1) 8 90.00 (-1/1)	(17)	00000	.120		.510	450	705	1.500	20,000		
(FI) = 45.00		3.0									
(FI) = 45.00 (×13) 0.000 2.321 3.150 4.275 5.802 7.875 7.875 (14) 0.000 2.321 3.150 4.275 5.802 7.875 7.875 (14) 0.000 2.52 3.43 4.719 5.791 7.259 20.000 (FI) = 6.009 2.532 3.43 4.719 5.791 7.259 11.615 (14) 0.000 2.542 4.800 1.590 1.580 1.580 1.590 1.580 1.590 1.580 1.590	(A/1N)	00000	1.354	1.838	2.494	3.384	4.594	765.7			
$ \begin{array}{llllllllllllllllllllllllllllllllllll$	(3)	00000	100	.100	907	1.010	2.520	50.000			
$ \begin{array}{llllllllllllllllllllllllllllllllllll$	E (14)	45.00									
(FI) = 45,00 (FI) = 45,00 (FI) = 0,000 0,000 (FI) = 05,000 (FI) = 00,000 (FI)	(×1/x)	00000	2,321	3.150	.27	5,402	7.075	7,875		•	
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	(v.j.)	00000	1024	.100	90p.	1.010	2.520	20,000			
(1.7) $0,000$ $2,532$ $3,433$ $4,719$ $5,791$ $7,259$ $11,015$ 1.7	1 2 5	45.08									
(14) = 65,00 (14) = 65,00 (14) 0,000 3,542 4,840 6,880 8,210 10,303 16,445 (15) 0,000 4,759 6,551 9,079 11,176 14,040 22,464 (14) 0,000 4,759 6,551 9,079 11,176 14,040 22,464 (14) 0,000 1,499 3,005 4,889 5,988 7,09 12,329	(×11×)	0.000	2.532	3.453	4.714	5.791	7.259	11.615	11.615		
(*/1) = \$5,00 (*/10) 0,000 3,542 4,840 6,880 8,210 10,303 16,465 (1) 0,000 ,075 ,150 ,280 ,440 ,700 1,580 (*/10) 0,000 4,750 6,551 9,079 11,176 14,040 22,464 (1) 0,000 ,073 ,140 ,280 ,440 ,760 12,369 (*/10) 0,000 1,499 3,005 4,6579 5,988 7,06 12,369	(N)	0.000	.070	150	290	077	001.	1,580	20,000		
(*11) 8,000 3,542 4,740 6,840 8,210 10,303 16,465 (11) 0,000 0,075 ,150 ,280 ,440 ,700 1,560 (11) 8 90,00 4,750 6,551 9,079 11,176 14,040 22,464 (11) 0,000 4,750 6,551 9,079 11,176 14,040 22,464 (11) 0,000 4,750 6,551 9,079 ,440 ,700 1,560 (11) (11) 0,000 1,499 3,005 4,657 5,988 7,700 12,329		65,00									
(FT) # 90.00 4.75 6.551 9.079 11.176 14.040 22.464 (FT) # 90.00 4.75 6.551 9.079 11.176 14.040 22.464 (TV) # 90.00 4.75 6.551 9.079 11.176 14.040 22.464 (FT) # 90.00 1.499 3.005 4.6579 5.988 7.106 12.329		000.0	3,542	T.	0.040	8.210	10,303	16,465	10,485		
(+1) # 90,00 (*/!*) 0,000 4,75° 6,551 9,079 11,176 14,040 22,464 (Iv) 0,000 4,75° 140 ,240 ,440 ,700 1,560 (Ff) = 90,04 (*/!*) 0,000 1,499 3,005 4,059 5,988 7,00 12,329	(11)	0.000	. 675	.150	Š	077	. 700	1,560	20,000		
(4.71^{10}) 0.000 4.750 6.551 9.079 11.176 14.040 22.464 (14) 0.500 .073 .140 .240 .440 .700 1.560 (+7) \pm 90.04 \pm 90.04 \pm 90.05 12.349 (+71) \pm 90.00 1.499 3.005 4.059 5.098 7.706 12.349		90.00									
(14) 0,500 0,073 140 240 440 700 1,580 (17) = 90,00 1,580 4,550 4,550 12,329	3	00000	4.750	. •	.07	11.176	14.040	22,464	22.464		
(+1) = 90,00 1,494 3,005 4,059 5,988 7,106 12,329		00000	.075	- 1	2	0 7 7 7	007	1,560	20.000		
(×/1×) 0,000 1,409 3,005 4,059 5,988 7,100 12,329		90.06							•		
		00000	1,494	3,005	250.2	5,488	7.100	12,349	12,329		

		U.S. VA	U.S. NAVY - ACMR PLATE	PLATFURMS	•	PLATFURM NU. 2	11 11 11	93.0 FEET - 50 YM STURM	0476	08/61/10
	95,00									
FINCE (N/IN) UEFL (IN)	0000	1.581	3,107	4. 4.44 7.50	6.314	001.0	13,001	13,001 20,000		
DEFIN (FE) H FORCE (F/1%) UEFL (12)	114.00	1.636	3,728	5,633	7.452	9.594	15,350	15,350 20,000		
	200.00	1,836	3,728	5,633	7,452	9.594	15,350	15,350		
	00000	. 022	160.	,250	. 410	00/•	1,580	20°000		
										·

and realization

PAGE 1 D.S. NAVY - ACHR PLATFURMS - PLATFURM NO. 2 - MAL 93.0 FEET - 50 YR STORM DATE 08/27/76		AT SUPPORTS OIFFERENCE PERCENT (IN. MAD) DIFFERENCE	-,00689	■ .05058	-,02472	#1000 ·	20000	7+527•	.10360	-2,46313	,25259	\$6145°	62000	.00432	918949	3,27949	-°32500	-,00957	5,000
- PLATEURM NO. 2 -	STRUCTURE DISFLACEMENTS DI		50007	P-04HP9	02472	# 1000 °	00500	24500	.02210	61251	•25259	Bosoo.	\$ 0 0 0 Z	-00m32	03512	643649	3 <500	- .00653	.000,55
YAVY - ACHR PLAIFURMS	STRUCTURE ACTIONS AT NUMLIMEAR	(84	-2,8624	-21,0041	-179.2508	-6100.4077	サラスト のこと	5,560.00/5	-3.0626	-441,4155	1700,1772	+011110+	505.2774	4775.6587	-1,9325	450.5142	-2240,4775	45057,4166	193,2478
	DESKEE OF	FREEDUM	-	~	- N	J .	'n	A	-	~	~	,	\$	٥	~	2	~	7	2
CYCLE NU. 1	NOVE INDEAS	JULAT NU.	1010	1010	1010	1010	9.01	0101	1011	1011	1011	1011	1011	1011	1016	1012	101	1016	1012

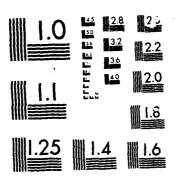
TRANSPORT DESIGNATION AT

08/27/76 PAGE PILE STRUCTURE
RESULTANT DISPLACEMENTS
UTFFERENCE PERCENT .0153 .0180 .0616 (IN, KAD) DIFFERENCE 0497 0012 - MML 93.0 FEET - 50 YR STORM 1465 .0001 • 0065 1146 ITERATION PILE DISPLACEMENTS AT SUPPONTS (IM, MAD) 2.51579 000000 33453 002468 00164 16838 22,41354 25202 00369 00264 - x - 1 1 1 0 8 - PLATFURM NO. DISPLACEMENTS AT SUPPORTS .10268 -2.30248 -.42145 .0246A .00059 2.37378 -.32416 00005 (INTAD) 40200 04500 000556 .00309 . NONLINEAK - ACHR PLATFURMS STRUCTURE ACTIONS
AT NONLINEAR
SUPPORTS (RIPS, IN-KIPS) 1.8970 414.9461 -2204.5070 51764.6422 2201.5730 -19,8225 -25,3295 -434.3169 1775.1283 4094,5851 -173,9035 -14104,6169 2424, 3496 -516ou.2u4/ 2 4 3 _ 0 Y > 4 2 U. S. FXELDUM DEGNEE VUNENE NO DOLL NO DOLL NO. 1010 1010 1012 1010 1010 1010 1010 1010 1111 1101 1014 1101 LIAD CUNDITION NO.

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U.S. NAVY - ACMR PLATFURMS	STRUCTURE ACTIONS AT NONLINEAR	SCENTINES (AIPS)	-19,2091	-25,0166 -179,0364	-1644.7776	-13447, 3670	\$678.4595	1,1203	-434,2205	1777,2096	-51465,9189	2612,1459	4200,9237	2,1739	6864.010	-2256,7381	52,42,3920	2554,5690	4134,0221
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DESIGN CALCULATIONS 93' MLW STRUCTURE EAST COAST AIR COMBAT MANEUVERING R..(U) CREST ENGINEERING INC TULSA OK SEP 76 27-771-95 CHES/NAVFAC-FPO-7614 F/G 13/13 AD-A165 689 NĽ UNCLASSIFIED



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08/27/76 PAGE DATE PILEASTHUCTURE
RESULTANT DISPLACEMENTS
01FFEWENCE .3962 (INSMAD) PIFFERENCE 0001 1000 U.S. NAVY - ACMH PLATFURMS - PLAIFURM NO. Z - MAL 93,0 FEET - 50 YK STUGM 0000 \$200 ITEHATIUNS 6000. 2000 0000 DISPLACEMENTS AT SUPPORTS ** 73924 ** 50948 ** 50170 17964 2.49310 2.49310 3.2442 0.00522 0.00452 .15523 (INTRAC) 51500 .03508 000138 .00378 , 25213 S U F F U X 1 DISPLACEMENTS AT SCHOOLS 2,44345 .10269 -2, 40906 -25213 85000 -. 50708 -. 10514 \$15000° (Instau) 15490 \$1200 .9U372 -. 12456 4.200. 13000 X Y Y Z I Z I Z I Z Y Y I S STRUCTURE ACTIONS AT WONLINEAN SUPPORTS (AIPS, InekIPS) 1775-0111 1775-0111 -51721-0070 2773-0109 2205,3004 52274,4129 2503,3296 -19.2720 -24.9274 -173.0026 2,1050 1.1200 4129.7030 -1654.9136 -15509.1537 3020,3709 FREEDUM DEGNEE NOVE INCAN JUL 1.1 1010 1010 1010 21.10 1010 CYCLE NO. 4

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PAGE DATE T - 50 YR STURM	PILE-STRUCTURE RESULTANT DISSIACEMENTS	UIFFERENCE PERCENT (IN.MAC) DIFFERENCE																		
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W.S. MAYY ACHR PLATFURMS - PLAIFURM NO. 2 - MML 93.0 FEET - 50 YR STORM	STRUCTUME ACTIONS AT MONLINEAR	SUPPLIKTS (KIPS, IN-KIPS)	-11,4675	-302,2844	042,0195	-27404.4217	-4663.0453	-103.4361	9,7478	カケンケ カウミー	1,51,2075	-57×53.2036	8413,7550	1019,6908	1757,	0 P O Z O B D D	-2649.4713	55469.2616	450.60#8	200,0196
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08/27/76 PAGE PILE-STRUCTUME RESULTANT DISPLACEMENTS UIFFERENCE PEPCENT (IN, MAD) DIFFERENCE .1384 0175 4000. \$600. 0739 ,0161 - PLATFURM NO. 2 - M#L 93.0 FEET - 50 YK STORM 5000 .0235 .0012 0260 I T E R PILE OISPLACEMENTS AT SUPPORTS (IN, MAU) -, 00014 05343 -1,48494 14926 00381 2.46769 -.37729 00000 -1.48466 -1.48466 -1.3592 000014 ******* STRUCTURE DISPLACEMENTS AT SUPPLIATS .37879 .1.48275 .14825 2.65411 2.57729 2.07709 00100 (IN, RAU) -. JUZZH -- 30000 * N C N L I N E A R U.S. NAVY - ACHR PLATFURMS STRUCTHRE ACTIONS (NIPS, IN-A [PS] -10.0825 -284.0475 -36040.4842 -36040.4842 200,4691 2655,0416 59746,9416 50,8560 10.6725 -293.7046 1044.8472 9916.2999 1156.3542 -54044 4534 -61.5492 295,3496 SUPPRINTS X X X L 0 FHELDUM DEGMEE YOUN INEAN U. 1.100 1010 1010 1001 1001 1001 1001 1001 LUAD CONDITION NO.

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08/27/76 PAGE PILE-STRUCTURE
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(IN-MAU) DIFFEMENCE .2522 , 1226 .0834 .0401 0057 NAVY . ACMR PLATFURMS . PLATFURM NU. 2 . MML 93.0 FEET . 50 YK STURM 8060 I TEKA TIONS .2808 \$000 .0003 0897 PILE DISPLACEMENTS AT SUPPORTS -00005 02378 2.31640 31791 0.0579 0.0279 00178 -2.55955 -2.55955 (IN, RAU) S C F C K T STRUCTURE DISPLACEMENTS AT SUPPERTS 02604 2.23.54 2.23.54 2.01.57 3.01.573 3.01.573 3.01.573 00000 -- 14273 .00261 E. 00178 .20551 (IMNHAU) . 39461 STRESEAN STRUCTURE ACTIONS AT MINLINEAR (RIPS, IN-KIPS) -, 8659 -267, 4230 395,0941 #2240,5202 #9715,6534 #2774,1159 -3,0355 -490/7-1907 -3005-4095 -5037-5441 17.9048 -1291,5250 13225,5073 1857,1100 -1.4144 1971,5867 SUPPORTS 0.8 FREEDUM DESKEE UF NONETREAR SUPPORT JOINT NO. CYCLE NO. 2

PAGE 9 0ATE 08/27/76	PILE-STRUCTURE RESULTART DISPLACEMENTS	IFFEKENCE PERCENT (In, pau) diffekence	A CANADA TANÀNA MANANTANA NA TANÀNA MANANTANA NA TANÀNA MANANTANA	•0252 •055e		•0005 0566			0134 0057	•		\$5000	,		.0434			.0002
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RESULTANT DISPLACEMENTS
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PAGE MAL 43.0 FEET . SO YM STURM 1 1 E K A 1 1 U N - x - 0 d d d U.S. NAVY - ACHR PLATFURMS - PLATFURM NO. 2 Ľ LOAD CUMUITIUN NU.

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54.	\$070°	8959.84	•			180.46	0000	20.
44.01	6.0543	5505.68	115,55	0000	-2.03	182.49	0000	20.
70.0	-, u 551	2451.00	•	•	-1.44	3	0000	٥ ٠
90.0	7×20.	40 H . 10	117.60	0000.	-	186.54	0000	00
50.05	0258	10.960-	119,63	0000	•	186.57	0100-	00
54.12	0101	-1207.34	Ð	0000	45.€	190.60	0000	00
4.75	-,0151	-1/57.75	٥	0000		192.62	0000	200
.17		74.002.1	~	0000	20.	٥	0000	00.
30.6		-1852,9U	127.74	0000	51.	190,68	0000	00
50.0	1200-	-1743.70	124.77	0000.	7.	198.71	0000	00.
46.54	6000	-1363,49	→.	0000	21.	200,13	0000	00
•	5000.	-1099-15		00000	-	0	0000	0 ° 0
-0		コト・ヘイメー		0000	0			

SISITE ANALASIO

	70111141			DE 61 E 1' T 1 (18			NO BE FOR THE SAME	
		13.4 CA 93.7	70	ALLEGA ALL TO	24.64.9	0110	MINDRAL TO	CALCAGO
•	- L	•	1	1		11224	4 110	MUMENT
	(1 ICHES)	(11-KIPS)	(611)	(Inches)	(IslakIPS)	(FT)	(INCHES)	(1Nex1PS)
	46.4.2.4	^	7	0000	17 1746	147.48	•	, ,
	1.5541	しつかさい	7 D 7 C 7	0000	17100	130.01		
	1.2739	-10045.9	72.49	8000	-84.79	141.93	00:00	5
	1,1573	-11162.95	75.42	1000	-23,82	143.96	0000	99
	1.0508	-4303.6d	77.05	• 0005	15,65	145.99	.0000	00.
	4114	2451.76	#0 * 7 L	4000	3H . 02	148.02	0000	90
	7257	4704,74	81.10	5000	64.74	150.04	000	00
	05/00	14429-15	H 5, 13	2000	48.63	152.07	0000	00
	.526	7521	H5.16	.0001	44,17	154.10	0000	00
	1957	25172.67	47.19	0000	36.46	150.13	-00000	00.
	1 to 1 to 1	20000	200	4.70	24 85	11	6	4
	1267	**************************************	27.16		10°00'	100.1	0000	
	1951	27.0.2.0	01.27	0000	13.78	162.21	0000	
	1 525	27141.42	95.46	0000	000	164.24	0000	
	2500	27.848.5	97.53	0000	4,02	160.24	0000	
			;					
	4970	10° 27' 10' 10' 10' 10' 10' 10' 10' 10' 10' 10	55.45	0000	7 ° 1 ° 1 ° 1 ° 1 ° 1 ° 1 ° 1 ° 1 ° 1 °	166.29	0000	3 : 0 •
	10104	1 1 2 1 2 2 2	60101			37.75	0000	000
		70 00 00 00 00 00 00 00 00 00 00 00 00 0	7: 70	0000	99	CC - 2/1		3 :
	0.10.1 	2025	107.45		30.0	170.40	0000	
; !	•							
	-, 0203	55.0400	104.49	0000.	-1.80	175.43	0000	٥٠.
:	0560	4157.54	111,52	0000	-1.44	180.46	0000	25.
	•• 0257	2502.44	115.55	0000.	-1.07	184.49	.0000	90.
	#020°-	9<1.71	115.57	0000.	-,73	184.51	0000	90°
1	6010.	-43,07	117.00	0000	-, 45	186.54	0000	200
	. v 1 2 9	-0/5.18	119.03	0000	45.	188,57	0000	70.
	C> C> C> •	-1655.47	121.05	0000		190.00	0000	200
	_ • €00 a b	-1184.1U	125.04	0000	.01	192.62	0000	00
11.00	7100.	-1191.47	125.71	0000	90.	194.05	0000	00. •
	0063	-1050.12	127.74	0000	90.	146.04	0000	00.0
50.03	0100.	00.000	129.77	0000	90	198.71	0000	00.
	1000.	07.5	131,00	0000	90 *	200,73	0000	7U.
	50000	-3c9.44	135,02	- >	90.	202.76	0000	30.0
	3 : 6 :			3 3 3 3	, d		,	

STRANTORIAN AND TO

		C.O. ZACY	ACHR PLATFURMS	- PLAIFURM NU.	4 - MML 93.0	FEET - SO YR STURM		
	DEFLECTIO:			DEFLECTION			DEFLECTION	
911 E	SURPAL LJ	BENDING	PILE	NUMBAL TO	BENDING	PILE	NUKHAL TO	BENDING
I		A STATE OF THE STA	LENGTH	PILE		LENGTH	PILE	MUMERA
	(INCHES)	(IN-KIPS)	(+1)	(INCHES)	(Sellyer)		(INCHES)	CRACKING
00.0	28.0	-33744.57	26.09	6000	#5.242 -	137.68	0000	.03
5.05	1.5403	-75544.71	70.47	5700	-177,20	,	0000	20.6
00.1	. 475	-19505.52	72,99	8000	10.00-	141.93	0000	10.
£0.0	1,1533	-12577,71	75.02	1000.	-27.09	143.46	0000	90°
. 111	1.0439	-5551,40	77.05	5000.	13,55	145,99	0000	000
1.10	4764	1550.25	74.08	4000	35,45	146.02	0000	3c
. 17	. C.D.	7247.41	61.10	£000°	47,19	150.04	0000	00
71.1	15.0.	13702,95	45.13	2000	44.59	152.07	0000.	20.
>> <	£072°	10721.43	42.16	.000	44.41	154.10	0000	000
. < 5	1500.	72742,42	87,19	0000	37,26	156,13	0000	000
χ.	9546.	25643.44	57°64	0000-	59.07	150.15	0000	33
42.50	7017	21300.42	41.54	0000	21,11	160.18	0000	•
24.35	2002	41.11.24	95.27	0000-	14.13	164.21	0000	90.
(b. 5)	.1304	27242,35	05.54	6000-	N. ES	164.24	0000	00.
9.59	1440.	25462,53	47.53	0000	4,23	100.46	0000	0.0
30.08	5000°	22/45.43	64.35	0000.	1,27	164.29	0000	70.
32.44	1110.	19547,30	101,58	0000	191	170.52	0000	00
24.47	0 ! 3 Z	16085.34	105.41	0000	-1.63	172.55	0000	30
96.0	.0101	12472,20	105.44	0000	-5.04	174.38	0000	90
25.5	-, 0255	4041,17	10/.45	0000	5002	170.40	0000	000
44.	0201	0075.40	104.44	0000	.1.H1	178.43	0000	70
45.53	64200	4306.97	111.52	0000	-1.46	140,45	00000	20.4
46.01	まるだい。	Z440.12	115.55	0000	-1.09	182.49	0000	3 0.
70.0	-, U2Ub	70°00°1	115.57	0000.	۲.	184.51	0000	70.
4.65	6010	14,35	117.00	0000	67	140,54	0000	0
80.05	0151	-454.54	119,03	0000.	-, 25	148.57	0000*-	20.
. 72	146.00	-1012.12	121.66	0000	0 1 2	190,00	0000	00
54.15	7010°	-1177.nl	125.04	0000	00.	192.62	0000	20.
. 17	57.0.	-1190,45	125,71	U 100°	90.	194.65	0000	00.
00		-1101,34	127,74	0000	90	196,08	0000	00.
.05	0011	452,75	124.17	0000	90.	198.71	0000	0.0
.0.	1000.	-111.51	131,00	0000	80	2	0000	00
•	<000°	05.065-	135,02	•	90.	202,76	0000	00.00

	į	U.S. KAVY	ACHR PLATFURMS	- PLATFURM NU.	. 2 - MML 93.0 FEE	T - 50 YK	STURM	
:	UEFLECTION			DEFLECTION			DEFLECTION	
FILE	POSMAL FU	DENDING	PILE	NUMMAL TO	BNIONBA	PILE	NURMAL TO	DENDING
F 1 5 5 1 3 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	r LE	FNB#O#	LENGTH	FILE	FZWET'S	FNGTH	PILE	HOMENT
() () () () () () () () () ()	. (834541)	(IN-KIPS)	(ET)	(INCHES)	(Salkevi)	(+1)	(INCHES)	(Selland)
5	4.6573	-54645.00	77.84	.0015	-645.87	137.88	0000	90.
2.03	K.0475	17300	74.07	. 0017	1453.90	139.91	0000	.05
0,3	00000°	-36252,14	72.49	•100.	-261,43	141.93	0000.	50°
r o	4.47.44	45.60045-	75.02	7100.	-118.07	3.9	0000	10.
111	K. U511	-12654.23	71.05	. 0011	-18,96	145.99		ng•
7.0	1.0579	-1027.50	ے •	6000°	~	148.02	0000.	20.
11.	1.0144	1000%-10	1.1	4000	~	150.04	000	10.
.19	1.5945	2027.00	H5.13	7000	48.54	152.07	0000	10.
77.	1.1410	29150.49	7.	2000.	۲.	154.10	0000	.01
52.	0.4740	36753.24	7.1	10001	v	150.13	0000	10.
p V	5441.	42204,69	7	0000°	1.5	158,15	0000	00-
24.50	1919	•	-	00000	'n	_	•	Ta ·
¿4,55	. 4740	40572.16	45.27	.0001	33,24	~	0000	200
36	. 5359	0960	95.50	.0001	~	3	0000	00
} - -	. 4200	47157.07	7,5	0000	ຼ	100,46	0000	000
17.	3451.	85. an. 55 a			5,17	158,29	0000	00°•
7.7.7	.679.	39216,08		0000	454	170,52	0000	3 0
4.47	\$ かぞの・	35490.30	103,41	0000.	-2.16	172,55	0000	30
94.0	2	27404.41	105.44	2	-3.50	174.38	0000	30
54.	0301	- 21413419	107,46	0000	-3,88	176.40	00000	000
45.55	0414	15403.14	104.40	0000-	-3.65	178.43	0000.	ى د
55	* C D O .	ゴエ # イア・コー	-	•	90.8	180.46	0000	00
4.01	50000	40.0000	115.55	0000	07.2	182.49	0000	æ•
7	2	3740.62	S	0000	-1.72	3	0000	00.
40	■•0545	1452,19	-	0000	\$1.15	186,54	0000	00.
3	0020.	-201,22	119.63	0000	00.	186.57	0000.	70.
21.25	+150.	-1207.91	121.06	0000	131	190.00	0000	70
7.5	1510.	-1462. \$1	123.08	0000	90.	192.02	0000	700
11	- c1c.	-2047.52	125.71	0000	• 00•	194.05	0000	20.
		-2017.11	127,74	0000	, 14	196.08	00000	000
20.00	651.0	*1441.1b	124.77	0000	• 16	198.71	0000	00
90.5	uo15	-1011, Hd	-	0000	~	200.73	0000	
10.30	70+0°	-1245.20	133.02	0000	.13	202.76	0000	20.
7	1100.	-461.20		0000	_		•	ī

The Assessment Madagast Transport

STRANIPILE ANALYGIS

THE RESIDENCE OF THE PROPERTY

						,		
1	WEFLEGILLS.			DEFLECTION			DEFLECTION	
7114	CI JARRON	5 TO 3 S	PILE	NURAL TO	BENDING	PILE	NURMAL TO	BENDING
<u> </u>	716	I CARAM	LENGIA	PILE	MUMENT	LENGIA	PILE	MUMENT
_(F7)	(THCHES)	(14-K1PS)	(FT)	(14ChES)	(I N=K I PB)	(FT)	(INCHES)	(INOKIPS)
30.	2.5704	-50404.11	Э.	7100	-542,43	137.88	0000	00
20.5	4.2103	-40566.05	70.47	5100	-345.90	139.91	0000	70
t.0.1	<.u5	-30555.98	72.49	.0013	-141.HC	141.93	0000	20.
٠ د ت	1.0707	-20501.87	75.02	.0012	-78.75	143.46	0000	*0
6.11	1.0931	-10171.74	77.05	6000	-2.03	145.99	- 0000	000
14	1,5000	-113,62	40°61	7000.	7.	140.02	0000	00.
2.17	1.5124	9117.13	91.10	5000.	64.54	150.04	0000	00
4.19	1.1544	16097.19	85,13	£000°	5.8	152.07	0000	. 01
27	6.454.8	25565.45	45.15	≥000°	72.40	154.10	0000	
۲۶.	1221.	31910.87	R1.19	1000	02,75	156.13	0000	70.
83.	. 5 \$ 1 u	30575,05	89.42	0000	50.35	154,15	0000	ju -
65	2054	253.5	91,20	0000	37.51	100.18	0000	•
24.55	. 3554	41401.17	95,27	0000	26.01	162.21	0000	20
, <u>5</u> 5	6/57	41355.42	95.30		16,33	164,24	0000	20
- 65	- 17UM	39446.11	97.53	10001	च र द	196.26	-6060	00.
50.41	.1017	50555,40	55.60	0000.	3.46	168,29	0000	3v*•
34.00	6443	\$2219.40		0000	- C 6	170.32	0000) C
24.47	• 0115	27164.21	105.41	0000.	#5.14	172.55	0000	3 c •
96.05	07.0	21,407,50	105.44	0000	63.09	174.38	0000	00
25.2	7700	16959.87	107 44	0000	•3,29	170.40	0000	70.
44.09	0571	12576,73	109.49	0000	-3.02	176.43	6000.	76.
٠, ا	22501	37.0752	111,52	0000	-2,51	180,45	0000	0 0
10	0375	5145.42	113.55	0000.	-1.92	ż	0000	no•
10.01	するかつ。	2035.40	115,57	00000	•	4.5	0000	00.
60.0	2027	70.50	117.00	00000	- N N	146.54	0000	00
	•.0220	-403.07	114.03	0000	22.	188.57	0000	00
~	2/10.	-1757.17	121.06	0000	-,22	190.00	0000	DC.
54.15		-1574.30	125.04	0000.	70.	192.62	0000	00.
. ' '	2233ª	11 x 00 x 1	125.71	0000	.01	194.65	0000	00.
(9)	0500.	-1749,10	127.74	0000	112	190,08	0000	000
50.0	25"O	-1504.20	129.77	0000.	7	196,71	0000	90°
60.5	*000°	-1315,79	131,60	0000	13	200,13	0000	0 1/2
56.00	1000	-1041.50	135.02	0000.	.11	202,76	0000	00
5	0100	-717.72	135.05	0000	90			

	PEFLECTION			DEFLECTION			DEFLECTION	
111	NOW ALL TO	AE NOING	PILE	NUKMAL TO	BRDING	PILE	NURAAL TO	DALCAME
LENGTH		35 B 2 4	LENGIH	ш	ROTENT	LENGTH	PILE	MOMENT
(61)	(I.4CHES)	(Salman))	(FT)	(INCHES)	(INEKIPS)	(F1)	(INCHES)	(IN-KIPS)
3	CC04-7	30.32433	20 1	5100.	65.4.76	117.88	0000	90
)	5.2479	57"55568"	70.67	0013	360.59	139.91		300
1000	- C\$CD.5	-24526.72	72.49	. 0013	-200.79	141.93	0000	70.
TO 0	1.901	-19477.24	75.02	.0011	89.26	145.96	0000	10
5.11	1.7157	• אמתון • מם	77,05	6000	-14,11	145.99	0000	200
0.14	1.547	Ox • 577	80.67	7000.		0	0000	00 **
2.17	1.5505	4409.25	H1,10	5000.	61.01	150.04	6000	20.
. 10	1,1554	10563.77	R 3, 13	\$0003		•	0000	90.
5.42	67.75	25743.37	A5,16	5000°	45° 84	154.10	0000	00.
57.0	0500.	31902,20	97,19	1000.	60.68	150,13	0000	00
46.44	50:4.	36430.25	29.65	0000	44,41	158,15	0000	20.
46.50	ξ 5η ς ,	14450.10	Ų	0000	37.45	150.18	0000	00
4,55	\$415.	41203,45	95.27	0000.	26,35	162.21	0000	00.
66.50	5465.	41187.78	~	10001	16.95	164.24	0000	99°•
3.54	.1407	39-29.20	97.53	0000	9.57	160.26	0000	00.
30.41	.1102	3	94,35	0060-	4.16	168,29	0000	000
32,44	\$050.	32504.30	101,58	0000	, 56	170.32	0000	3 0
77.7	2/14	27455.26	103.41	0000.	-1.61	172,55	0000	70.
Λ	5400.	24204.48	3	0000	-2.69	174.38	0000	90
34.54	. 0250	11292.42	107.45	0000	-3,01	176,40	0000	nu a
55.0	1.0347	12713.40	104.49	0202.	18.5-	178.43	0000	00
64.23	0578	0/35.11	111,52	0000.	-2,41	180.46	300	00
٥	5080	7	115.55	0000	-1.68	182.49	0000	0 °
۰	u 529	2914.33	115.57	0000	-1,35	164.51	0000	<u>ا</u> د د
60.0	0.0000	~	117.60	00:00	22.	180.54	0900	06
09.3	6225	10.002-	114.63	0000.	•.52	180.57	0000	00.
۷.1۷	. 117:	-1100.11	121,06	0000	-, 25	190.00	0000	0°
54.75	0125	-1555, 14	165.08	0000	07	192.62	0000	70.
6.17	■. U750	-1724.71	125.71	00000	70.	194.65	0000	00.
0.00	\$ \$60	-1592.70	127.74	0000	010	190.68	0000	00
₹ 0•0	0929	-1551.39	124.17	0000	15	198.71	0000	00
~5.65	••0.11	-129H.97	Ď	0000	, 12	200.13	Э,	00.
76.37	1000	-1050.69	135.02	0000	.10	405.76	0000	20.0
5	0000	11.00	2	0000	4			

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		C.S. SAVY . D	ACHR PLATFURMS	- PLATFURM NU.	2 - MWL 93.0	FEET - SO YM STURM		
	UEFLECTION_			DEFLECT 1UN			DEFLECTION	
7 I.t	CI JANADY		PILE	NUKHAL TO	BENDING	PILE	NURMAL TO	BENDING
1 Jo	r I L E	AOKE2T	LENGIA	r I CE	RUMENT	LENGIA	PILE	MUMENT
(F1)	(INCHES)	(5dTx=1.7)	(FT)	(INCHES)	(1 v=x 1 PS)	(11)	(INCHES)	(INOKIPB)
50.0	1.4503	• \$2501.35	44.44	0100.	*206.25	137.88	0000	\$ 0.
- P	1,5541	1250/0.40	74.07	0	150.00	159.41	0000	20
£ 0.3	1, < 311	/185.7	72.49	8000°	-73.24	141.93	0000	6
£ 0.0	1.11/9	-10000-42	75,02	⇒	-15.45	143,96	0000	00
6.11	\$444	-57/11.90	77.05.		21.29	145.99	6000-	00
71.	20.0		80.44	#000 *	7	146.02	0000	06.
_	1247		81,19	2000		150.04	0000	٠,
÷	2/40*	14507-11	85.13	1000		152.07	0000	•
6.42	775.	かん。つかでかし	45.16	• >	7		0000	2
52.		25045.59	A7.19	0000	35.20	150.13	0000	
	3		ת מ	•	2	1	6	i
	, ,		77	> =	•		•	
	1961		95.27	0000		162.21	0000	20.
65.0	1261		95.50	0000	: -	164,24	0000	20
95.8	\$6742	2467H,27	97.53	0000	3.47	166,26	0000	•
30.41	5150.	21 HOS. 02	55.46	0000	.71	160.29	0000	0: • •
52.44	40104	10514245	~	0000	•	174.32	0000	00
71.15	#/00·	15190.35	103.41	0000.	-1.85	172.35	0000	90°
ુ ે ૧	. c.1 d 3	11×53.AU	7.50	0000.	~	174.38	0000) (•
٥٠٥٥	1450.	6772,45	107.46	0000	-2.08	170.40	0000	0.0
46.55	1050.	0.104.75	104.49	0000-	æ	178.43	0000.	20.
14.35	+420.	5-15-50	111,52	0000	-1.42	140.46	0000	20
•	₽\$ 2 0 • -	2034,54	115.55	0000.	٥.	126.49	0000	0.0
20.0	ファーニ・・	724.18	115,57	0000.	04	184.51	0000	0°
40.0	1010.	-178,76	117,60	0000	2 7 7	180,54	0000	200
70.7	•. U164	-701.44	114,03	00000	•	188.57	0000	00.
71.5	(503 ·	-1004.32	171,06	0000	7.0	190,60	0000	00
54.15	20an••	-1204.65	123.08	0000.	₹0.	۰	0000	00.
`.	*. € 5 5 5 5	21.25.11-	125,71	0000.	10.	194.65	0000	00.
0.00	1200.	€		0000	60	140.68	0000	၁
\$ 0 ° (€000°	5.07	7.67	0000	7°	_	0000	00.
•	0 ::0	-744.58	131.60	9	80	400.13	Ö	30
4.30	9000	-560.30	135.82	0000.	00.	~	0000	20.
	0000	7 0 000	1	0:00	4			•

スローストのストルはMIPのストルのののではMIPのストルトルにはMIPのストルルのないできました。

PAGE U.S. NAVY - ACMY PLAIFURMS - PLAIFURM NI. 2 - MIL 93.0 FEET - 50 YN STURM

11 08/27/76

以

CEPLEE 134	SENUING	PILE	NORTH TO	BENDING	PILE	NUMBAL TO	BENDING
71.6	MUMERA	LENGTH	: Laj	-ZUECE	LENGTH	PILE	MUMENT
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1.5542	-30454.00	27.00	6000.	-252,15	137.88	0000	£0.
۵	-24514.07	70.47	6000	-147,50	139.91	0000	20
1.1764	-17713.71	72.49	#000°	00.60-	141.43	0000	70
1.0559	-11243,27	75.02	9000	-14,23	143,96	0000	20
, 5¢	-4407.27	77,05	5000	20,05	145,99	0000	00.
\$5	1741.43	74.08	£000.	59.65	146.02	0000	00
1221	1405.27	M1,10	2000	47.14	150.04	0000	00.
.0154	15517.35	H5.13	.0001	45.85	152,07	0000	30
5405.	11407.40	1.0	1000	41.82	154,10	0000	90.
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12 -08/27/76 PAGE DATE

BENDING MUMENT (IN-KIPE) 2020 30000 2 2 2 2 2 33333 DEFLECTION NURMAL TO PILE 0000 00000 00000 0000 0000 000000 0000 0000 0000 0000 (INChES) - SO YK STURM PILE LENGTH (F.L) 190.00 194.02 194.05 137.88 139.91 141.93 143.96 50.02 50.04 52.07 54.10 58.15 160.18 162.21 164.28 166.29 170.32 172.35 174.39 178. 43 130, 43 134, 50 134, 50 136, 50 200,73 - MML 93.0 FEET DENDING MUMENT (LY+KIPS) -444.20 -265.72 -125.34 20.44 77.61 78.61 78.06 9 7 7 9 0 -675.59 55.01 51.63 50.03 50.03 50.03 - PLATFUPH NU. DEFLECTION. NUMBAL 111 PILE 00000 2100 2100 2100 2100 2100 50000 LINCHES) 000 - ACMR PLATFUHMS 114.03 121,66 125.08 125.71 PILE LENGTH (FT) 105 41 105 41 107 44 109.49 111.52 115.55 70.47 74.00 77.10 77.10 84.62 41.64 45.67 95.50 129.77 131.40 135.62 135.65 17,60 C. S. NAVY 102504 102504 64751 39251 1547,80 20152.05 37644.88 32200.00 20553.34 (IN-KIPS) 40560,50 44245,54 46245,36 46581,05 -1032.07 -1040.00 -1900.00 BENDING PUMENT -5651H,05 -45205.11 -35926,79 -1510,21 10109.FI 17801.02 28206.65 35145.37 -22674.44 -11459.40 15107,70 -1761.Hu - 553.55 DEFLECTION NUMMAL TO PILE .00.50 1.7345 1.5363 1.5584 1.1569 C 266 . 7 7 20 . 0 9 4 9 . 4 5 7 4 . 3 3 1 8 .1427 -. u207 -. u208 .0100 (INCHES) PILE LENGTH (FT) 36. 24 36. 24 30. 24 30. 25 30. 25 - 10 mc 10.00 m 20.63 22.50 24.55 45.05 18.53 60.60 56.09 56.72 56.73 50.03 00.000 90.00

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STRAN MERRER DETAIL REPORT

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STRAN SERBER DE "ARL REPORT

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STRAN AEMBER DETAIL REPORT

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STRAN MEMBER DETAIL MEPORT

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STRAN MEMBER DETAIL REPORT

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STRAN ARABER DETAIL REPORT

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STRAN AFREFR OFTAIL KEPORT

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THAN AEMBER DETAIL REPORT

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STRAN MEMBER DETAIL REPORT

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STRAN MEMBER DETAIL REPORT

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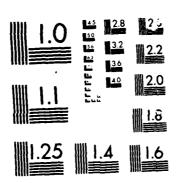
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PAGE 66 DATE 08/27/76 U.S. NAVY - ACMR PLATFUMMS - PLATFUMM NO. 2 - MML 93.0 FEET - 50 TH STURM LUAND, CONVITTION NO.

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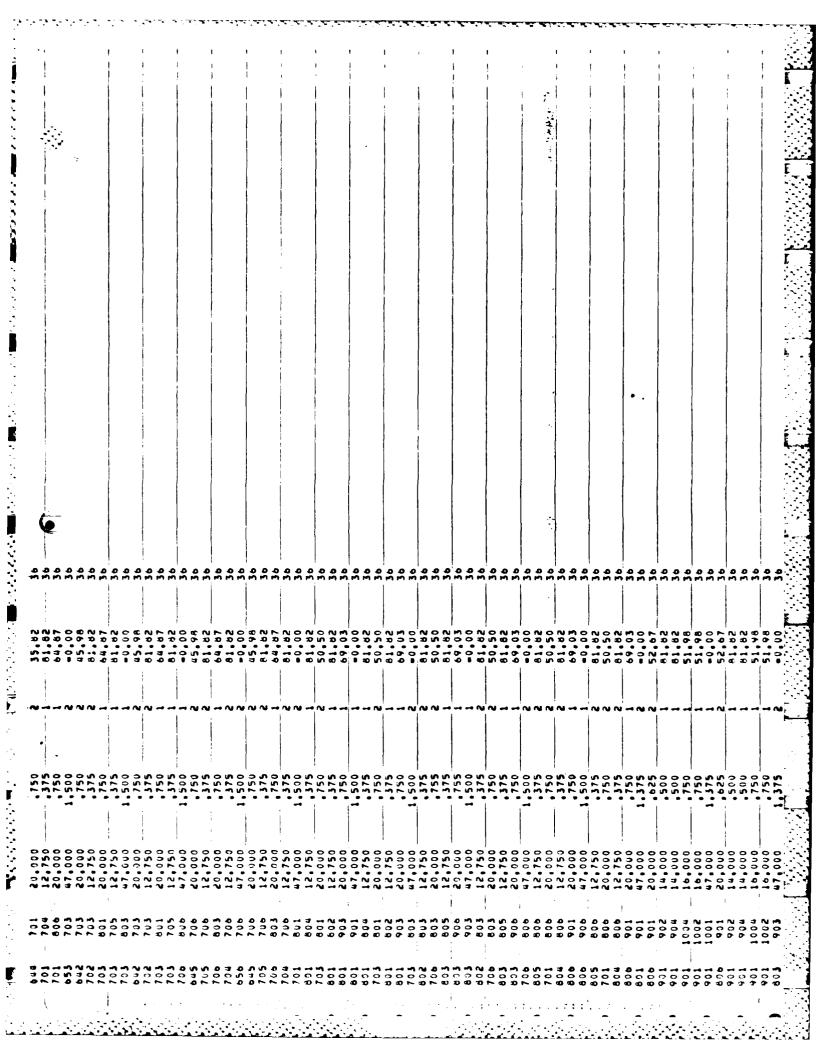
APPENDIX B.3

SAPCHK - Primary Joints

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SAPCHX - CREST DFFSHORE, INC. STRUCTURAL POSTPROCESSOR SYSTEM

PUNCHING SHEAR CHECK FUR . S.NAVY 27-771-01 93 FT MLH STRUCTURE PRIMARY JOINTS

ALLOWABLE PUNCHING BHEAK	6,625	9.625	9.625	9,625	9,625	9,625	9,625	9,625	9,625	9,625	9,625	9,625	9,625
CALCULATED PUNCHING SHEAR	2,785	4,213	1.020	679.	3,881	897.7	5.645	5.102	4,393	6.476	4.742	4.755	4.011
1 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	1.793	1,828	2.73	1.800	2,044 5,008	2,278	9.047	607.8	1,113	1.524	2,720	3.232	11,527 5,699 10,921
AXIAL	. 521 . 032	.570	432	. 403	.056 6.709	7,267	1,149	1,092	1,021	1,214	5,855	8.107	6.143 4.781
TITCKZEGG	1,250	1.250	1.250	1.250	1.250	1.250	1,250	1.250	1,250	1,250	1.250	1.250	1.250 500 500
OIAMETER	30.00	30.00	30.00	30.00	30.00	30.00	30.00	30.00	30,00	30.00	30.00	30.00	30.00 12.75 12.75
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SAPCHK - CREST OFFSHORE, INC. STRUCTURAL POSTPROCESSUR SYSTEM

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ALLUMABLE PUNÇHING SHEAR	4.166	67056	9.150	9.216	2.00 2.00		9.101 9.101	8,784 8,784	6,863	6,177	9,101	6,739	6,733	0,733 0,733
PUNCHING SHEAR	7.037	7,685	6.198	4,619 6,296	5.542		6,269 5,241	7,591	962.9	7.725	5,241	6.531	909.9	5.082 0.527
E S S	15,237	14,968	14.270	14.027	17,429	15,556	7.010 5.712	17.506 10.690 6.705	15.557	19,365	15,567	17,619	4000 E	5,124
/* = 5 T R	127	1.230	1.209	1.057	4.050	1218	2 0 0 E	4.429	1,637	1.920	3.024	2,101	1,520	1 . t . t . t
TITCKZEGO	1.250	1.250	1.250	1.250	1,250	~ ~	,750 ,750	1.250	1,250	1.250	1,250	1,250	1.750	1.000
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SAPCHK - CREST OFFIGHURE, INC. STRUCTURAL PUSTPROCESSUR SYSTEM

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ALLCWABLE PUNCHING SHEAK	6.733	6,733	0.733	6.733	8.733 8.735 8.733	8.733	8,733	8,733 8,733 8,733	6.733 6.733	6.733 8.733 8.733	3 3 3 3 4 5 4 5 5 5 5 5 5 5 5 5 5 5 5 5	8,681
CALCULATED PUNCHING SHEAR	7.527	3,066	6.807		7.692 3.756 3.132	6,606		7.527 3.410 3.066	6.807 2.824 4.134	7.692	1.872	2,749
E S S (BENDING	9.637	0,173	991	4,495	10,714	6,097	• •	4,632	7 - 7 4 2 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	0.947 10.714 4.235 6.311	6,001 3,740 7,073 5,571	8 0 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
AXIAL	401.00 401.00 401.00	140	6,939	• • (8.082 4.951	4 98 98 98 98 98 98 98 98 98 98 98 98 98		M	1.298 6.939 4.240	4,530 8,062 4,931	6.919 853 5.059 9.357	4,341 3,844 3,759
TICKNESS .	1,750	10	1.750	00 7	1 000	1.750		750	1,000	1,750 ,750 1,000	1,750 750 1,000	1,750
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OINTS	ALLOWABLE PUNCHING SHEAK		8,566	9,566	8,566		8.663	8,663	6,663		8,733	6.733	8,733	-	8,733	8,733	6,733		8,733	A. 711
-01 93 FT MLM STRUCTURE PRIMARY JOINTS	PUNCHING BUNCHING		2,107	6.051	7,781		3,251	5,403	7,376		1.872	4,963	7.024		2,749	0,715	7,077		2,107	100.4
LM_STRUCTURE	SS /* -S T R E S S/ CALCULATED AXIAL BENDING PUNCHING BMEAR	6.330	5,114	9,179	6.762	8,432	2,863	9,142	0.00	5.419	3,740	7,073	5,571	7,751	2,857	7,782	5,746	5.578	5,114	0 1 10
01_93 FT.M	AXIAL	7.408	690	5,619	9,751	4,672	5,055	990.7	00006	5.472	.853	5,059	9,357	2,983	3.844	3,759	9,289	5.802	690	014
	THICKNESS	1.750	150	150	1.000	1,750	.750	.750	1.000	1,750	,750	150	1.000	1,750	.750	,750	1,000	1,750	.750	750
PUNCHING SHEAR CHECK FOR - SONAVY _27-77	DIAMETER	00.64	16.00	16.00	20.00	48.00	10,00	16,00	20.00	00.87	16,00	16.00	20.00	48,00	16.00	16,00	20.00	00.84	16.00	
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SAPCHA - CREST OFFSHURE, INC. STRUCTURAL PUSITHOCESSUR SYSTEM

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771-01 93 FT HLM_STRUCTURE _ PRIMARY_JOINTS_	CALCULATED PUNCHING SHEAR	4.607
MLM_STRUCTUR	E S S	4.962 7.972 7.879 6.162
1 93 FT.	AXIAL	0.714 3.309 8.716 9.244
/YZ7-771-0	TITCE	1.750 750 750 1.000
PUNCHING SHEAR CHECK FOR S.NAVY 27-	DIAMETER	24 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
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4.607	25		.78	907.9	36		7	7,852	78		56		0		3	6,802	. 25		88	2,901	.21	5.		204	353	987.	134		40	2,894	. 32	1
7.972	16	6	9,914	ξ9.	9.	73	6.	9,781	80.	7	8.607	88	9	3 0	7.972	٠.	٦.	5	73	4,772	4	55	•	Э-	~	s.	₹.	•	~	4.750	Ŀ.	0
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SAPCHK - CREST OFFSHORE, INC. STRUCTURAL POSTPROCESSUR SYSTEM

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AXIAL		5.518	12,162	,525	10.184	079	4,145	8.612	6,319	2,463	3,358	12,407		10.167	.389	4.535	•	1.960	•	2,757	70107	16,102	10,184		5,842	•		70)	3,863	,255	3.367	96.
THICKNESS		1.500	75	37	75	1.500	37	75	750	50	375	75	37	S	0	~	•	372.	•	50	.375	C	150	0	75	37	.750	-	1.500	.750	77	
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SAPCHK - CHEST OFFSHORE, INC. STRUCTURAL POSTPHOCESSUR SYSTEM

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ALLOWABLE PUNCHING BHEAN	.93	.93	7.930	93	1	.93	5	7,930		.93	.93	7.930 7.930		0	7,930	93	93		693	֓֞֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֡֓֓֓֡֓֓֓֓֓֡֓֓֡	7.000		. 93	7,930	.93	93	: :	7.040		.93
CALGULATED PUNCHING SHEAR	3,832	2,333	6.037	m		2,097	1,571	1,483		•	1,919	5, 80 8 5, 81		2,140	1.32	3,003	066		€,	1 0	1,378		•	1,571	•	•	i i	1984 1987	35	.83
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SAPCHK - CREST OFFSHORE, INC. STRUCTURAL POSTPHOCESSUR SYSTEM

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PUNCHING	1.779	10 0 10 10 10 10 10 10 10 10 10 10 10 10	1000 1000 1000 1000 1000	1000 1000 1000 1000 1000 1000 1000 100	1,779		4,607
BENDING	1.000 0.000 0.000 0.000 0.000	W 4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	MONT BUNDA B	40 4 40 40 40 40 40 40 40 40 40 40 40 40	N 1 E W 8	N. C. S. S. S. S. S. S. S. S. S. S. S. S. S.	1,237
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SAPCHK . CREST OFFSHURE, INC. STRUCTURAL POSTPHOCESSOR SYSTEM

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THICKNEGG	1.500	375	1.500	375	1,500	375	1,500	- 10	1 3000 1750 1750 1750	1,500 1,375 1,750 7,50	1 500 1 750 2 750
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BAPCHK - CREST OFFEHORE, INC. STRUCTURAL POSTPHOCESSUR SYSTEM

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PULCE AND BURGER	7.040	7.950	7,930	7.930	7,930	7.930 7.930
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AXIAL	2.671 3.750 3.150	.218	5.586	3.647	3,316	1,371 9,383
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	THICKNESS /= 48 T R B S 4 1 CALCULATED AXIAL BENDING BUNCHING BURGAR	JOINT LUAD BRACE DIAMETER THICKNESS /= +S T R E S S = -/ CALCULATED NUMBER CASE NUMBER PUNCHING BEEAK STALL BENOING PUNCHING BHEAK STALL BENOING BHEAK STALL BENOING BHEAK STALL BOING S. 5078 S. 750 4.954 2.078 1.350 5.240 5.240 5.240 5.240 5.240 5.240 5.240	JOINT LUAD BRACE DIAMETER THICKNESS /- "S T R E S S CALCULATED NUMBER CASE NUMBER PRICHING AXIAL BENDING PUNCHING BHEAR 101 801 804 12.75 3.750 4.954 2.078 1.382 801 802 12.75 3.750 3.712 2.059 1.382 801 903 20.00 .750 3.712 2.059 1.382 801 903 20.00 .750 .218 5.851 2.062	JOINT LUAD BRACE DIAMETER THICKNESS /= "3 T R E S S = " CALCULATED NUMBER CASE NUMBER PRINCHING STATE BENDING PUNCHING STATE STATE BENDING PUNCHING STATE ST	JOINT LUAD BRACE DIAMETER THICKNESS /- "3 T R E S S - " CALCULATED NUMBER CASE NUMBER PRINCHING SHEAR BOIL BOIL BOIL 12,75 3,750 4,954 2,078 1,362 2,078 1,362 2,059 1,362 2,078 1,362 1,362 2,059 1,362 1,362 1,362 1,362 1,362 1,362 1,362 1,362 1,362 1,362 1,362 1,362 1,362 1,363 1,3	JOINT LUAD BRACE DIAMETER THICKNESS /- "S T R E S S - " CALCULATED NUMBER CASE NUMBER PRINCHING SHEAR AND 1.500 2.671600 2.078 SHEAR 703 801 20.00 750 .136 0.410 2.078 1.362 0.410 2.078 0.136 0.410 2.078 1.362 0.410 2.078 1.362 0.410 2.078 1.362 0.410 2.078 1.362 0.410 2.078 1.362 0.410 2.078 1.362 0.410 2.078 1.362 0.410 2.059 1.362 0.410 0.375 1.363 0.410 2.059 1.362 0.410 0.4

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GAPCHK . CREGI OFFIGHORE, INC. SIRUCTURAL PUSITACCESSUR SYSTEM

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CALCULATED PUNCHING SHEAR	4	9	2,016	28		ň	93	0	4		6.87	40.40	9) 	i	7	-	7.240			•	1.565		 		4.824	.73	79.		.87	4,425	9
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AXIAL .	3.271	11.019	3,229	10,338	1,671	3,727	10,159	1.284	9,2,8	5,313	٦,	•	•	•	5	. 36	~ ~	205		240.0	6.170	4.434	5,860	~	2	•	.77	\$ 45	\$6.	44.	6.311	66.
THICKNESS	1,500	750	375	.750	0	,375	2	~ 1	7.5	1.500	37	96/1	2 5	•	1,500	3		75		2 5	- 4	375	75	0	,375	75	3	75	20	37	150	3
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SAPCHK - CHEST OFFSHORE, INC. STRUCTURAL POSTPROCESSOR SYSTEM

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GAPCHK + CREGI OFFGHORE, INC. GIRUCIURAL PUSITEROCEGIO GYGIER

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ESS.	1.098	6	15	.73	20.0	58	2,129		•	2.477	•	•	73	0.5	77.	7000	0.5	90.	. 27	750.4	9	6	.22	.54	1.082	20:	S
AXIAL AXIAL	9100.9	23	7.2	. 29	35.	2	_12,028_	~	3	5.019		5.	34	15.	2	6.650	8	.53	67	5.077	5.1	77	9	÷.	1,923	7 6	25
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BAPCHK - CREBT DFFBHORE, INC. STRUCTURAL POSTPHOCEBSOR SYSTEM

ALCUMABLE PUNCHING WHEAK	7 . 447 7 . 447 7 . 447 7 . 447 7 . 447	7.447 7.447 7.447 7.447	7.447 7.447 7.447 7.447 7.447	7.447 7.447 7.447 7.447 7.447	7.447 7.447 7.447 7.447	7.007 7.007 7.007 7.007
CALCULATED PUNCHING BHEAK	2 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	2,560 3,916 3,920 5,705	4 W W W W W W W W W W W W W W W W W W W	10.01.1 10.01.1 10.01.1 10.01.1 10.01.1	4.670 3.124 4.028 6.391 3.843	2,5560 3,920 3,920
HE G G I I	11. 12. 12. 13. 13. 13. 13. 13. 13. 13. 13. 13. 13	20 20 20 20 20 20 20 20 20 20 20 20 20 2	11. 11. 12. 13. 14. 14. 14. 14. 14. 14. 14. 14. 14. 14	40 40 40 40 40 0 0 40 50 0 0 0 0	M E M E C M C C C C C C C C C C C C C C	1,426 9,552 1,570 1,659
AXIAL R	13.550 7.550 7.750 13.750 6.66	11 11 11 11 11 11 11 11 11 11 11 11 11	1, 499 5, 077 8, 077 8, 626 13, 544 6, 774	14. 20.00 20	7.0582 4.0544 7.7544 13.0068	357 334 1074 7568
THICKNESS	4 4 5 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	500 500 500 500 750	2 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	5000 5000 7500 7500	5 W 75 W 75 W 75 W 75 W 75 W 75 W 75 W
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SAPCHK - CREST OFFSHORE, INC. STRUCTURAL PUSTPHOCESSUR SYSTEM

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PUNCHING BHEAR	Na. a	2,393	3,893		5.069	4 1 1 4 3 6 8 4	5,624	•	4,150	3,279	1.689	N. 637		5,10	3,564	3.027	5,454		3,943	4 1 4 6	6,545	5,893		5,069	4010	2,004	. C 2 4
E 8 9/ C BENDING	. 170	3.282	3,367	194	206.0	3.610	3.011	907	1.095	2.400	1,603	2.00	.822	22	3,296	1.608	3.527	.471	1385	4,223	7.00	3,367	096	4,945	7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	•	***
AXIAL AXIAL	12,393	3,600	13.602	099.7	11,251	6,765	11.993	121.4	12,331	966.9	3,243	9 9 9	4.511	11.008	6.926	ζ.	11.626	.530	12.393	000	0000	7,143	1595	11,251	7.288	12.272	100
THICKNESS	1.375	200	75	_	20	9 0	750	17	· N	S	3 5	750	3	3	Š	9 1	750	_	N	0	> W	750	~	9	000	2 LF	
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SAPCHK - CREST OFFSHORE, INC. STRUCTURAL PUSTPROCESSOR SYSTEM

CALCULATED ALLOXABLE 'PUNCHING PUNCHING SHEAR SHEAR	154	1,689 7,447 5,759 7,447 3,637 7,447		3,564 7,447			2,992 7,062	. 178 7		4,773 7,062	555	2,627 7,062		8.541 7.062 4.149 7.062		6,539 7,062	•	7002	
E C C C C C C C C C C C C C C C C C C C	1.095	2,369	1,008	3.296	2.650	125.6	6.089	6,339	2.353	4.007	157	5.496	1327	1,599 3,620	667	690	,326	3.601	484
AXIAL	12.331	13.243 6.843	11.068	0 r	11,919	11.828	57	.358	366	5,875	.001	570	, 352	5.477	.592	9.569	000	6,530	564
UTHUX NEW PERSON	1.175	750	1,375	500	750	047.	\$29	.625	1,250	929	1.250	629	1,250	. 625 . 625	1,250	625	1,250	625	1.250
OIAME TER	47.00 20.00 14.00	4 4 4 4 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	20.00	000		•	20.00	٥.	46.00	20,00	900	20,00	00.98	0000	46.00	20.00	90.00		46.00
S S S S S S S S S S S S S S S S S S S	400 P	4 906 6 1005 6 1004	6	00		0		100	100	0	•	0		1 1004		3 1005	•	3 1005	
1 S	20 0		80	00	0	Š	8	0	100	100	001	00		000			5		

PUNCHING SHEAK CHECK FOR & S.NAVY. 274714-01. 45 FT HIN.STRUCTURE. PRIMARY JOINTS. BAPCHK - CREST OFFSHORE, INC. STRUCTURAL POSTPROCESSUR SYSTEM

SEL RZOZOZZA BODOSZA KONTORO

NEW	C A 9 E	BRACE NUMBER	DIAMETER HETER	NO WELL	AXIAL	/* +6 T R E G G CALCULATEDAXIAL BENDING PUNCHING BHEAR	CALCCLATED PUNCHING PUNCHING	ALLOIABLE PUNCH NO BIRAK
903 1003 1003	100	1002 1003 1003 1005	20°00 20°00	1,250	5.40 5.40 5.40 5.40 5.40 5.40	2.373	3.919	7,062
406 1006 1006	•	1004 1006	20.00	1,250	9.275 8.738	3.521	6.036	7.062
908 1008 1008	7 011	1004 1006	46.00 20.00 20.00	1.250 .625 .625	10,324	3.593 3.468	6.446	7,062
906 1006 1006	1001	1004 1006	20°00 20°00 20°00	1,250	.564 9.257 8.988	2,512	5.697	7,062
900 1000 1000	•	1004-1006	20°00 20°00	1,250	10.523	3,313	6,651	7.062
END_OF_JOINT_CHECK	END OF JUINT CHECK							

APPENDIX B.4

SAPACHK - Secondary Joints

STRUCTURAL PUSTPHICESSUR SYSTEM THETA ANGLE JOINTS CHECK, PUNCHING SHEAR FUR TUBULAN MEMBERS SECONDAMY STARIZENU - CHEST OFFSHURE, INC. STRUCTURE THICKNESS 3 1 10,750 SAPCHE INTOC

NUMBER 12 12 12 13 14 15 15 15 15 15 15 15
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SAPCHK - CREST OFFSHURE, INC. STRUCTURAL PUSIPANCESSUR SYSTEM

w .												
PUNCHABLE BUNCHABLE BUNCHABLE BUNCHABLE	7.914	7.914		7.914	7.748	7.914		7.914	7.914	7.914	7.251	7.082
CALCULATED PUACHING SHEAR	4.052	3,773	366.	7,168	3,950	2,713	,	3.680 6.578	2,935	3,721	2,660	3,109
20 00 00 00 00 00 00 00 00 00 00 00 00 0	5.842 6.150 4.652	5.595 5.495	5,738	0 . L 0 . L 0 . L	5.784	2.435	2.571	4.608 8.712	2,691 2,594 5,524	2.736 4.726 8.834	5.426	4.5 20 4 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5
AAIAL	2,148	5.460	786	2,256	1.217	7.466	5.550	2,254	8.242 2.200 1.623	6.033 1.449 2.183	11,527 .541 1,632	13.856
TAICKE MOO	.50u .365	500	005	. 365 265	.500 .365 .365	. 500 365 365	500	365	. 500 . 365 . 365	2500 250 250 250	505. 505.	365
OIAMETER	10.75 10.75 10.75	10.00	> ⊅:	10.75	10.00	16.00 10.75 10.75	10.00	10.75	16.00 10.75 10.75	10,00	10,75	10.75
BRACE BUNKHER	2 504	504	n i	2 20 2 0 0 2 0 0 3 0 0	2 504	2 505 4 505	1	2 505 4 505	2 505 4 505	505 4 505	2 504 4 505	2 50¢
C A SI E	5.00	200	20	y y	50,	5000	•	y y	\$ \$000	5000	9 00 00	200

SAPCHE - CHEST OFFSHURE, INC. STRUCTURAL PUSTPHOCESSUR SYSTEM

ALLU*ASLE PUNCHING SHEAN	7,407	7,320	0.000.00 0.000.00		98.8 98.8	3.000 0.000 0.000	0.80.0 0.80.0 0.80.0	0 8 8 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	8,8%U 6,6%U	>&& \$ \$ \$ \$	U 4 60
CALCULATED	2.430	2.856	5,060	3,291	5,000	3,396	4,151	2,251	4,026	2,22A 5,214	722
E S S E E S E E S E E S E E S E E E E E	5.064 5.440 5.504	2	7.098 5.22 5.072	6.348 3.350 3.418	7.504	5.480 5.480	3,404	6.099 6.136 5.628	5.480	6.291 2.124 5.103	4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
/= =S T H AXIAL	10.651	12.806	027	3.358	4.555	4,184 ,756 ,601	1,258	1.554	1.235	1,663	3,374
THICKNESS	\$00 \$45 \$45	200° 468 268	375 365 365	. 375 . 365 \$48	375 395 365	365	.375 .365	365	375	\$75 \$65 \$65	375 385 365
UIALETEH	10.00	10.75	10.75 10.75	14.75 10.75 10.75	16.75	10.75	10.75	10.75	16.75	12.75 10.75 10.75	12.75 10.75 10.75
SERVER SE	502 205	504	704	704	704	708	705	705	705	705	704
CASE B	50 Z	205	702	7 702 792	705 705	702	702	702	732	702 704 704	702
JUNEAU DE	3 0	7 0	¥	טע	' U	70	٥,	,	S.	\$07	3

SYSTEM
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CALCULATED ALLUMABLE PU.CHING PUNCHING SHEAK SHEAK	2,246 8,880 5,41 K, KRU		2.124 8.880 5.290 8.880	5,272 8,880 260 8,880	3,101 8,880 2,809 8,880	5,267 6,880 ,751 6,880	3,129 B. BBU	3,9423 6,880 3,9423 6,880	2,097 8,880 5,240 8,880	3,762 6,630 3,749 6,690
ESS/	2.080		1,483	7.540 5.501 5.70	3,247	5.490 5	6,557 5,295 5,178	2.019 3.445 3.679		2.197 5.075 3.074 3
THICKLESS /= =S T R	1,767	0.00	355 1,740 355 .735	.375 1.108 .355 1.056 .555 .046	3,056 3,65 3,65 5,573	1,510 1,054 1,054 1,044	3712 365 365 545 545 562	275 3.647 247 1.054 265 1.014	. 375 1.374 . 355 . 606 . 355 1.175	. 575 3.229 . 565 1.047 . 565 1.004
NIMER OTAMETER	12.75 704 10.75 705 10.75	4 v	704 10.75 705 10.75	805 10.75 804 10.75	805 10,75 804 10,75	805 10,75 804 10,75	805 10.75 804 10.75	12.75 805 10.75 805 10.75	805 10,75 805 10,75	12,75 405 10,75 805 10,75
LUAU	7 702	8 70	702	\$ 0.5 \$ 0.5	408	20 p	20 € 20 €	0 0 0 0 0 0 0	7 808 804	# P P P P P P P P P P P P P P P P P P P
RADED DACED	104 766 764	704 706 704	704 706 704	801 HUZ 802	801 802 808	801 80 <u>2 6</u> 02	001 60¢ 60¢	805 805	SON SON SON	33. 805 305

SAPLHA - CHEST UFFSHURE, INC. STHUCTURAL PUSTPHUCESSUR STSTEM

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ALLUMANLE PUNCHING SHEAK	2000.00	9.69.0 0.69.0) 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0#8.5	9.341	9,341	9.341
CALCULATED PUJCHING SHEAR	. 638 5.004	2.055	4,671	1.966 4.886	.514 u.374	1,840	659
THICKNESS /* *9 T R E S S * */ CALCULATED THICKNESS /* *9 T R E S S * */ CALCULATED SHEAH	4.665	2.189	4,024	2.084 1.094 4.422	4,72¢ 170 0,450	2.042 3.042	4.710
ANIAL	0.093	4,739 570 1.175	4.436	4.771	905.5 07.0	1,959	5.019
THICKNESS	375 365 365	. 375 365 305	\$75 \$45 \$45	375 305 448	500	395.	.50c
DIAMETER	12.75 10.75 10.75	12.75 10.75 10.75	12.75 10.75 10.75	12.75 10.75 10.75	14.00	10.75	10.75
NACE NUMBER	80 th	808 808	304 305	3 5 C 2	906	904	906
	508 B04	209	305	70 P	908	206	700
CASE	٥	,	I.	0	•		ar:
JUINT BUTHER	9	700	700	700	705	706	206
CHURCH CH	0 1 2 0 0	404 404	979 7 79	\$ 0 th	206 106	401 408	401 405

9,341

3,796

5.514 949 5.576

3.239 079 915

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908

707 707

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106

9,341

2,551

4.440 5.443 5.001

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5.124 441 1.004

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SAPCHA - CREST OFFSHURE, INC. STRUCTURAL PUSTPAUCESSUR SYSTEM

ACCOMABLE PUNCKING SHEAR	9.341 9.341	9.341	9,341 9,341	9.341	9,341	8.616 6.733	8.816 8.733	8,810 8,735	8.610 6.735	6,810 6,733	8,816 8,734
CALCCLATED PUNCHING SHEAK	1,570	2,496	1,667	2.693	1,734	4.803 1.692	1.744	1.459	1,593	4.621	1,377
ESS	4,510 2,125 4,731	2.045 3.166 5.076	1,203	2.77¢ 5.494 3.843	4,408	2.180 4.07¢	3,064	2 4 5 5 4 5 5 4 5 5 5 5 5 5 5 5 5 5 5 5	2,424 3,185	2.160 2.271 3.620	2.654
AXIAL	6.450	7.640	7.286 .603 1.068	646.	5924	131	3,546	6.610 0.017	3 457 011 051	5.602 6.762	3.768
89 37 X DI E F	.500 .365 .365	500.	\$00 365 365	5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	500	875 750 375	750.75	. 875 150 375	150 150 175	475 750 275	. 875 150 875
UIATETER	14.00	10.75	10.75	10.75	10.75	24 00 10 00 14 00	30,00	2 2 2	10,00	2 C C C C C C C C C C C C C C C C C C C	10,00
2 Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z	4 0 0 0 a	506 7	6 905 905	506 2	2 905 4 905	1 1602	1 1002	1002	1 1002	\$ 1002 2 1005	5 1002 2 1005
CASE	705	705	404	2000	406	901 1002	1001	1001	100	1002	403

SAPCHK - CREST OFFSHURE, INC. STRUCTURAL PUSITACCESSUR SYSIEM

ALLU-ABLE PUNCHING SHEAK	d.810 d.733	8.816 6.735	6,616	6.810 6.733	6.733	8.810 8.738	8.816 6.733	8.810 6.735	6.416 6.735	6.810 8.733	0100
CALCULATED PURCHING SHEAK	4,669	1.211 1.568	7,428	7.014	7,509	6.992 1.402	8.413	7.710	A.016	7,326	4,780
S S/ BENDING	. \$15 2.479 5.25u	4.020	1.496	1.07/	1.002	1,921	2,988	2,415	2,500	. 50d 2.516 1.702	1,014
AAIAL AAIAL	5,328 6,651 .758	3.470	13.553 13.553	3.908 12.216	5.354 13.258	3,506 11.846 .450	5.229	5.965 12.264 .848	13,327	6,046 11,928	7,150
THICKNESS	. 150 . 150	875 120 127	875 750 375	875 . 750 375	475 150 275.	875 750 375	. 475 . 750 . 375	. 875 	875 750 375	. 475 . 750 . 750	875
UIAMETER	24.00 10.00 14.00	16.00	16.00	24.00 16.00 14.00	10000	10.00	24,00 16,00 14,00	24.00 14.00 14.00	10.00	24.00 16.00 14.00	10,00
ARACE NUMBER	405 1002 1002 1005	903 1002	403 1005 002 1005	903 1005 1002 1205	903 1005	405 1005 1002 1005	906 1005 1004 1005	900 1005 1004 1005	406 1005 004 1005	906 10U5 004 10U5	901 1904
LUAD ÇASE	10	•	4	-	æ	5	٠	-	10	•	•

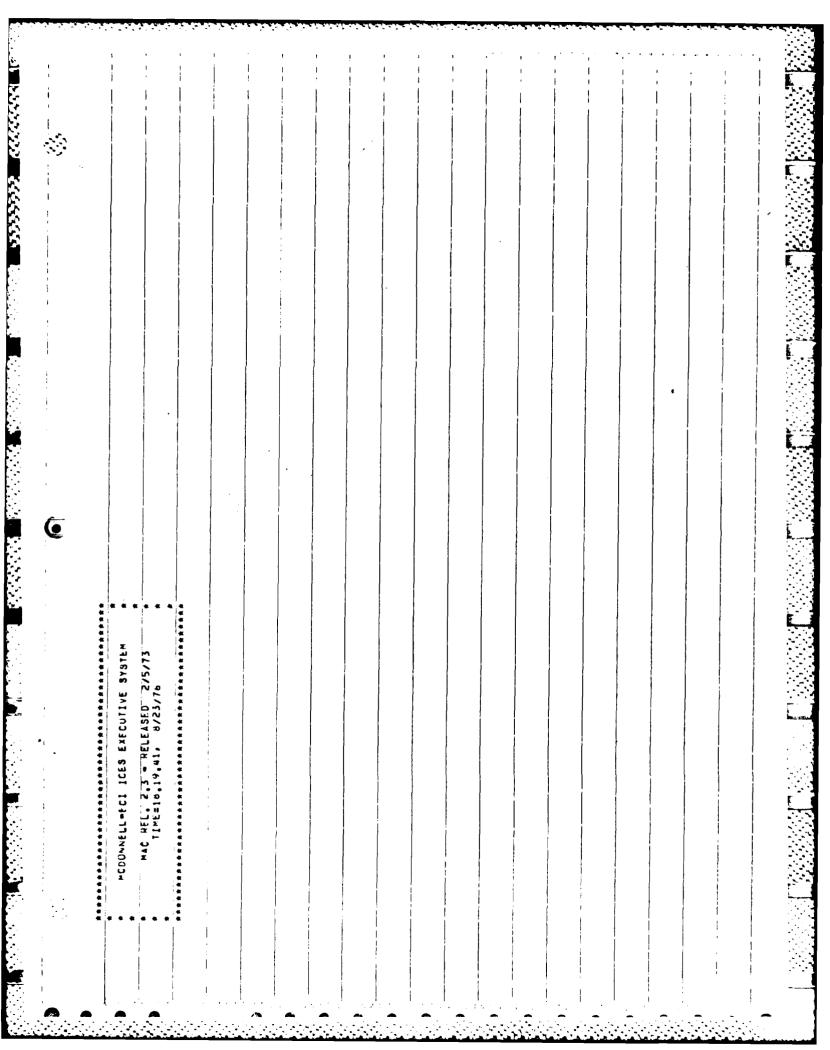
SAPCHA - CREST OFFISHUFE, INC. STRUCTURAL PUSTPRICESSUR SYSTEM

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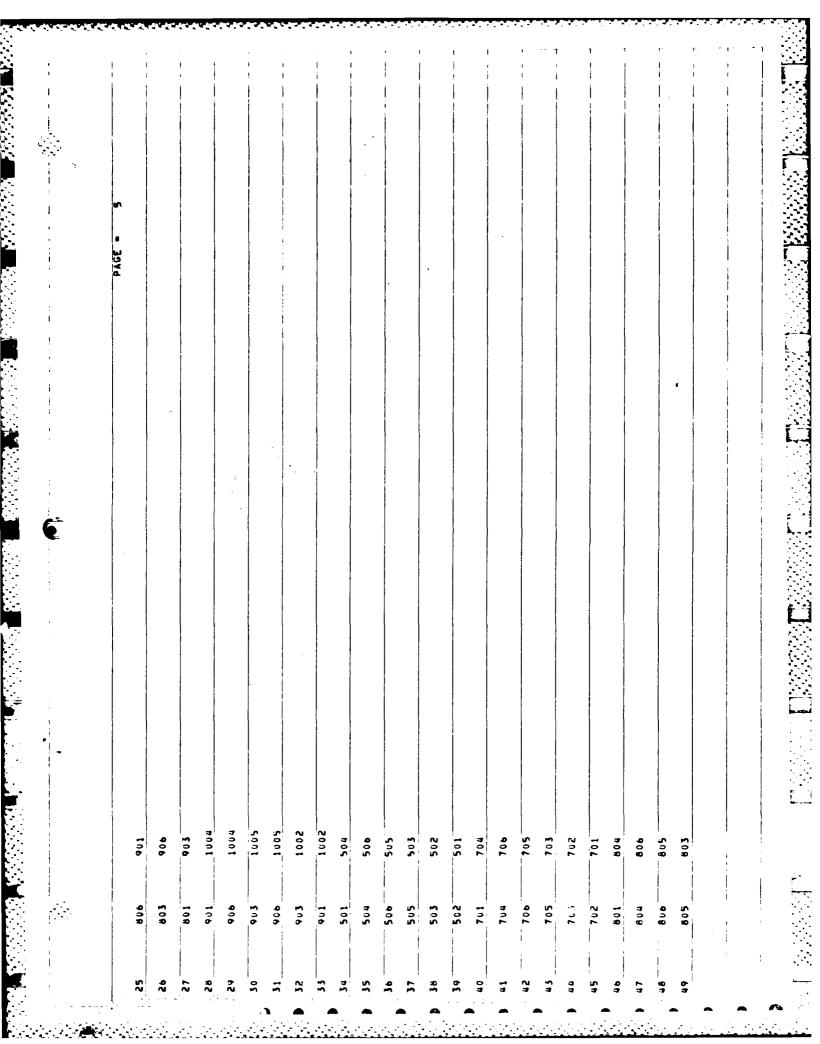
004 1004 7 901 1006 16.00 .875 1.516 .639 7.321 1902 1004 14.00 .750 12.04 2.400 1.205 1902 1004 1004 14.00 .375 .017 4.590 1.681 1002 1004 0 901 1004 14.00 .375 .455 1.315 1006 1004 0 906 1004 10.00 .875 5.51 2.486 1.315 1006 1004 0 906 1004 10.00 .475 6.176 2.406 1.315 1006 1004 0 906 1004 10.00 .475 6.176 2.504 4.857 0.600 1006 1004 0 906 1004 10.00 .475 6.475 2.504 4.857 1.153 6.000 1006 1004 0 906 1004 10.00 .475 6.475 2.504 4.857 1.163	VOTER PURER	C & S.E.	NITHER	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	COSNUCTO	AXIAL BE		DING PUNCHING	PUNCHING
7		:	:					CHEAR	SHEAR
9 401 1004 10,00 475 5,000 7,321 1,205 1002 1,004 1,205 1,002 1,004 1,205 1,002 1,004 1,205 1,002 1,004 1,000 7,50 6,879 2,453 4,768 1,002 1,004 10,00 7,50 7,50 11,495 2,750 7,420 1,002 1,004 10,00 7,50 7,135 2,700 5,029 1,004 1,005 1,007 7,135 2,700 5,029 1,004 1,005 1,000 7,50 7,135 2,700 5,029 1,004 1,005 1,000 7,50 1,096 2,107 7,135 2,004 1,000 1,0	21 1004 1004		-	24.00	875	3,516	4634		
1002 1004 14.00 .475 .026 .830 4.768 9 01 1004 10.00 .475 .017 .029 1002 1004 10.00 .475 .455 .017 .029 1002 1004 10.00 .475 .455 .455 .2.76 1002 1004 10.00 .475 .455 .485 .485 1004 1005 1004 10.00 .475 .6.178 .485 1004 1005 14.00 .475 .6.178 .485 1004 1005 14.00 .475 .6.178 .485 1004 1005 14.00 .475 .6.178 .485 1004 1005 14.00 .475 .6.178 .485 1004 1005 14.00 .475 .489 2.574 1.163		•		16.00	05/	12.057	2.400	7,321	8.616
9 901 1004 10.00 .875 .017 4.690 1.681 9 901 1004 10.00 .875 .017 4.690 1.681 1002 1004 10.00 .875 .3.270 .754 1.315 0 900 1004 10.00 .875 5.551 2.700 5.029 1004 1005 14.00 .875 6.178 2.107 7.138 7 24.00 .875 6.178 .945 1004 1005 14.00 .875 6.178 .846 1.138 8 900 1004 10.00 .750 11.984 2.107 7.132 8 900 1004 10.00 .750 .875 6.178 .805 900 1004 10.05 14.00 .375 8.540 .485 1004 1005 14.00 .375 5.540 .805 900 1004 1005 14.00 .375 5.540 .805 900 1004 1005 14.00 .375 5.540 .805 900 1004 1005 14.00 .375 5.540 .805 900 1004 1005 14.00 .375 5.540 .805		10	1004	14.00	375,	392	3,004	1,205	8,733
9 24,00 875 3,270 5,780 1,681 1,681 1,002 1004 14,00 3,750 11,495 2,756 7,420 1,012 1004 14,00 3,750 3,750 2,453 44,20 1,012 1004 14,00 3,750 3,750 2,756 7,420 1,012 1004 14,00 3,750 3,750 2,485 1,315 2,700 5,029 1,004 1005 14,00 3,750 11,984 2,107 7,132 3,945 1004 1005 14,00 3,750 3,848 3,870 3,600 4,857 1,004 1005 14,00 3,750 5,894 2,504 4,857 1,163 3,750 3,750 3,574 1,163 3,750 3,750 3,750 3,574 1,163 3,750 3,750 3,574 1,163 3,750	1 1004 1004	Ð		24.00	678	.026	0830		
1002 1004 14.00 ,575 ,017 4,590 1,681 9		3	01 1004	10.00	750	6.819	2,453	4.768	8.810
9		10	02 1004	34.00	375	.017	069.7	1,681	8,733
+01 1004 10,00 .750 11,855 2,756 7,420 +02 1004 14,00 .375 3,651 2,74 5,029 +03 1004 10,00 .575 7,133 2,488 1,138 +04 1005 14,00 .575 11,984 2,107 7,132 +04 1004 10,00 .375 3,848 .870 .600 +04 1004 10,00 .375 5,540 .4857 +04 1004 10,00 .375 5,540 .4857 +04 1005 14,00 .375 5,540 .4857 +04 1005 14,00 .375 .723 2,574 1,163 +04 1005 14,00 .375 .723 .2574 1,163 +05 1004 1005 14,00 .375 .723 .2574 .163	1 1004 1004	5		24.00	.675	3.270	\$75		
1002 1004 14.00 .475 .455 3.252 1.315 900 1004 10.00 .475 7.133 2.700 5.029 1004 1005 14.00 .475 0.178 2.478 1.138 1004 1005 14.00 .475 0.178 2.107 7.132 8 900 1004 10.00 .475 5.540 .475 0.00 8 900 1004 10.00 .475 5.540 .475 0.00 900 1004 1005 14.00 .475 5.540 .475 0.00 900 1004 1005 14.00 .475 0.875 2.504 1.163	! ! !	3	01 1004	10.00	750	11.455	2,750	7.420	6.810
7 90c 1004 10.00		2	02 1004	14.00	.375	. 455	3,454	1,315	8,733
1004 1005 14.00 ,575 7.133 2.700 5.029 1004 1005 14.00 ,575 ,741 2.466 1.136 7 24.00 ,875 6.178 ,945 1004 1005 14.00 ,575 848 ,870 ,600 8 906 1004 10.00 ,875 5.540 ,485 1004 1005 14.00 ,750 6.298 ,2574 1.163	4 1006 1004	٥		24.00	.875	5,551	.574		
1004 1005 14.00 .875 .741 2.488 1.138 7		•	00 1004	10.00	150	7,133	2.700	5.029	5,416
7 906 1004 10.00 , 750 11.964 2,107 7,132 1004 10.00 , 750 11.964 2,107 7,132 1004 10.00 , 375 848 , 876 , 600 4857 906 1004 10.00 , 750 6,892 2,504 4,857 10,4 10.05 14,00 , 875 6,298 , 855		10	5001 70	14,00	\$78.	141	2.488	1,138	6.733
906 1004 1005 14,00 3/5 848 2,107 7,132 1004 1005 14,00 3/5 5,540 4,870 6,000 8 906 1004 1005 10,00 3/5 6,540 2,504 4,857 1004 1005 14,00 3/75 6,298 ,955	4 1006 1004	^		24.00	.675	6.178	945		
1004 1005 14,00 ,5/5 ,848 ,878 ,600 8 906 1004 10,00 ,750 6,892 2,504 4,857 1004 1005 14,00 ,375 7.3 2,574 1,163 9 24,00 ,875 6,29R ,852		3	1004 90	10,00	150	11,984	2,107	7,132	8.816
8 906 1004 16,00 ,875 5,540 44,857 1004 1005 14,00 ,375 6,298 ,2574 1,163 9 54,00 ,875 6,298 ,852		2	5001 00	14.00	٠٤/٤.	848	.670	009.	8,733
906 1004 10,00 ,750 6,892 2,604 4,857 1004 1005 14,00 ,375 ,723 2,574 1,163 9 9 9 9 9 9 9 9 9 9 9 9	4 1006 1004	10		24,00	478	5,540	242		
1004 1005 14,00 ,375 ,723 2,574 1,163 9 24,00 ,475 6,29A ,952		3		10,00	057.	6.892	4007	4.857	8.816
462.9 ch. 00.45 c		10	5001 10	14.00	375	.743	4.574	1,163	8,733
	1006 1004	>		24.00	578.	6.29R	558.		
1004 16.00 .750 11.835 1.645 6.916		•		10.00	9.5.0	11,635	1.045	916.9	8.616
1004 1005 14.00 .375 .855 .654 .529 8.735		=	- 7	14.00	1375	1805	4004	529	8,735

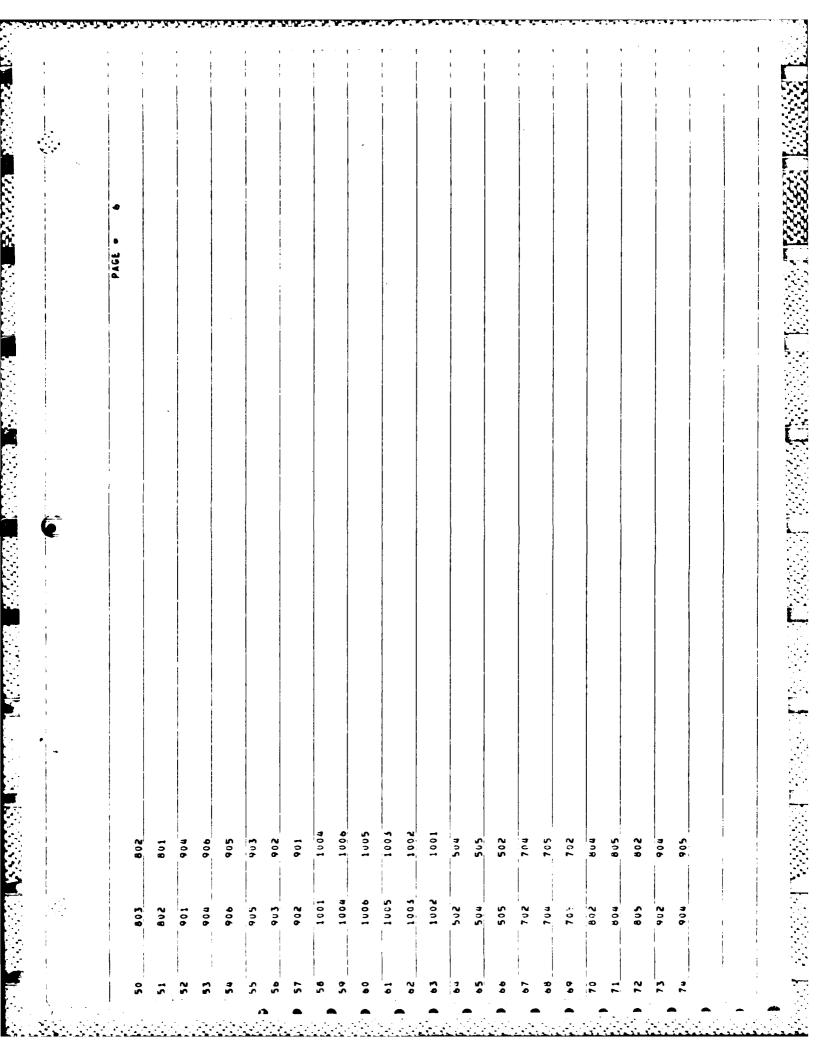
APPENDIX B.5

LIFT ANALYSIS



703 -28.79				
	0 0.	32,37	PACE • 3	
		• •		!
705		10,19		
700 -20,09	09 -18.76			
. 801 0.	15,51	• 5		
802 -1.14	11.40	19.68		
803 -5,24	.0 .05	36.86		
*0 *08		•		
805 -1.62	-11,20	19,43		
806	-22,51	• 0		
901 27.09	26.41	o o		
91.55 59.	13,21	22,79		
903 23,29	65	45,58	8	
904 27,09	• 0	•		
602506	13.21	22.79		1
900 27.09	09 -26.41	• 0		}
1001 54.18	18 50,31	• •		
1002 - 52,01	01 15,16	20,16		
1003 49.83	85 0.	52,31		
1000 54,18	18 0.	•		
10.52 5001	01 -15,16	26,16		
1006 54.18	18 -30,31	• 0		
1101 58,20	20 30,89	•		
1103 53,76	10 0.	53,25		
1166 _ 58,20	20 -30.69	• 0		





506	905							
1004	1004							
1004	1005							
1005	1002							
JUINT RELEASES								
103 903 HUN X HUN Y HUN Z	Z HOH A	FUR	\					
TIES PR	MEMBER PHUPERTIES PRISMATICS							
AX 254,27 IX 13	IX 136171,28	7.7	17 68085,64	12 68085	999,5908	8 2	2836,90 \$2 8	2856,90
A 214.41 IX 1	1x 111095,36	17 5	17 55546,68	12 55546	546.68	1	SY 2363,69 SZ 2	2363.69
7 TO 15 - TA AX 71.47 IX	Ix 50995,38	1 71	17 18497.69	12 18497	997.69	> 0	804,25 82	804,25
0 18 - AX 175,73 IX 8	88047,86	1 4	IY 44023.93	12 44023	1023,93	3 7 8	SY 1914,08 SZ 1914,08	1914,08
0 21 • AX 66,71 IX	5962.60	>	2981,40	21	2961,40	8	298,14 52	296,14
22 TO 27 58 TO 65	3574,08	1	1787,04	12 1	1767,04	>	178,71 52	176,71
26 fu 33 - 1x	1787.12	\	895.56	12	893,56	> 3	111,70 82	111,70
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7 TU 9 FUH Z GLU UNI FH " -728,92 LA 0,833 LB 1,0	
10 TO 12 FUR 2 GLO UNI FH 728,92 LA 0,0 LB 0,10	
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13 TO 15 FUR Z GLU UNI FH010 77 LA 0.0 LH 0.13	
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13 TO 15 FUR Z GLO UNI FR P -597,42 LA 0.95 LB 1.0	
16 TO 18 FUR Z GLO UNI597 42	1
19 TO 21 FCR 2 GLU DAI	
22 TO 27 FIR Z GLU UNI # -129,32	
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34 TO 39 FUR Z GLO UNI # -62,77	
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58 TO 63 FOR Z GLO UNI " -129,32	
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706		-1.12	3.17	-75.03	224.57	-55.35
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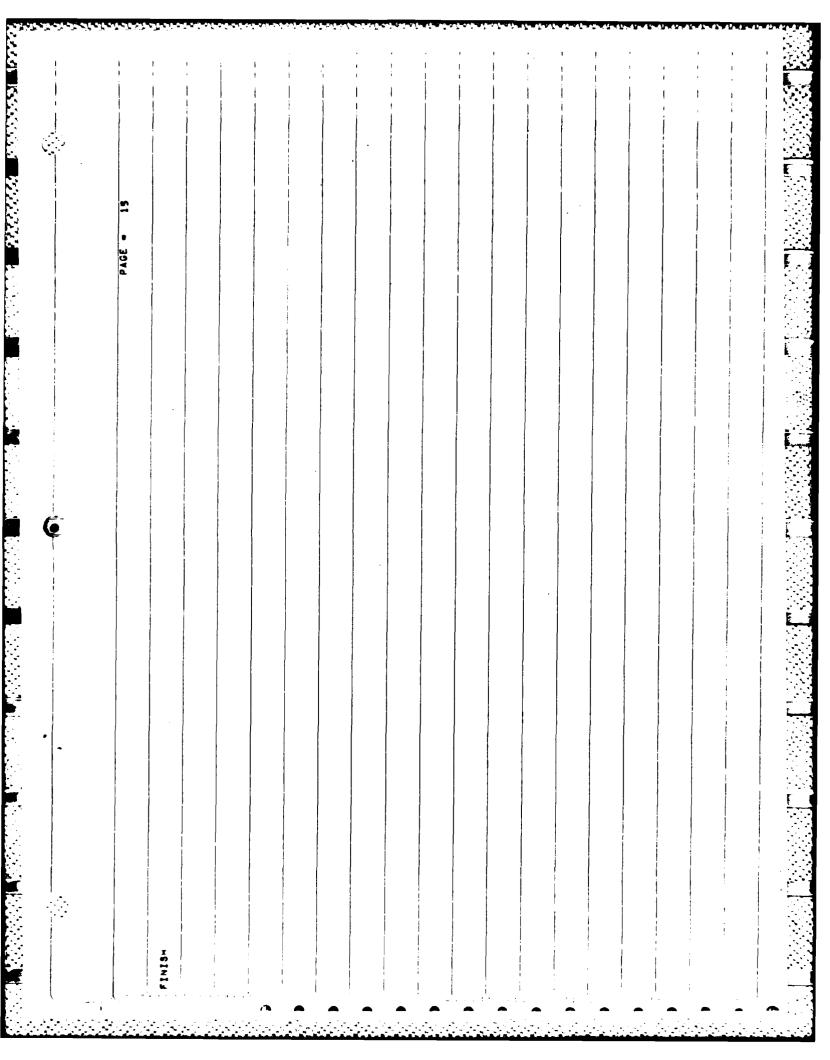
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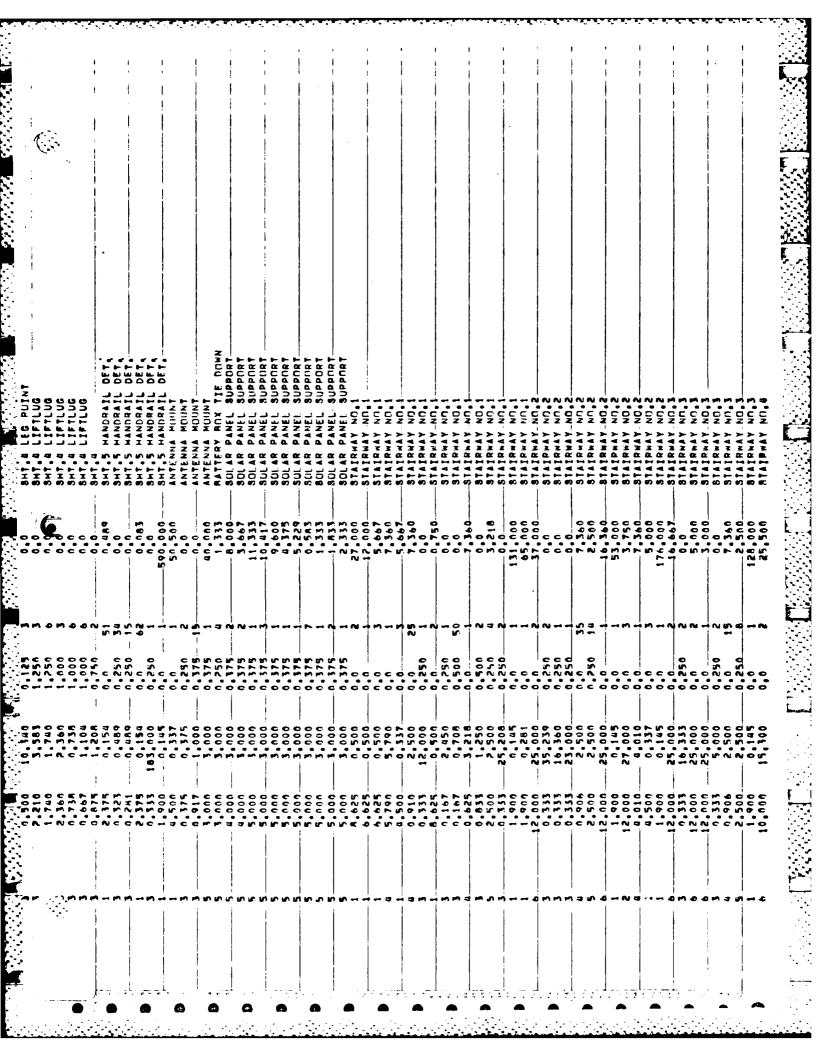
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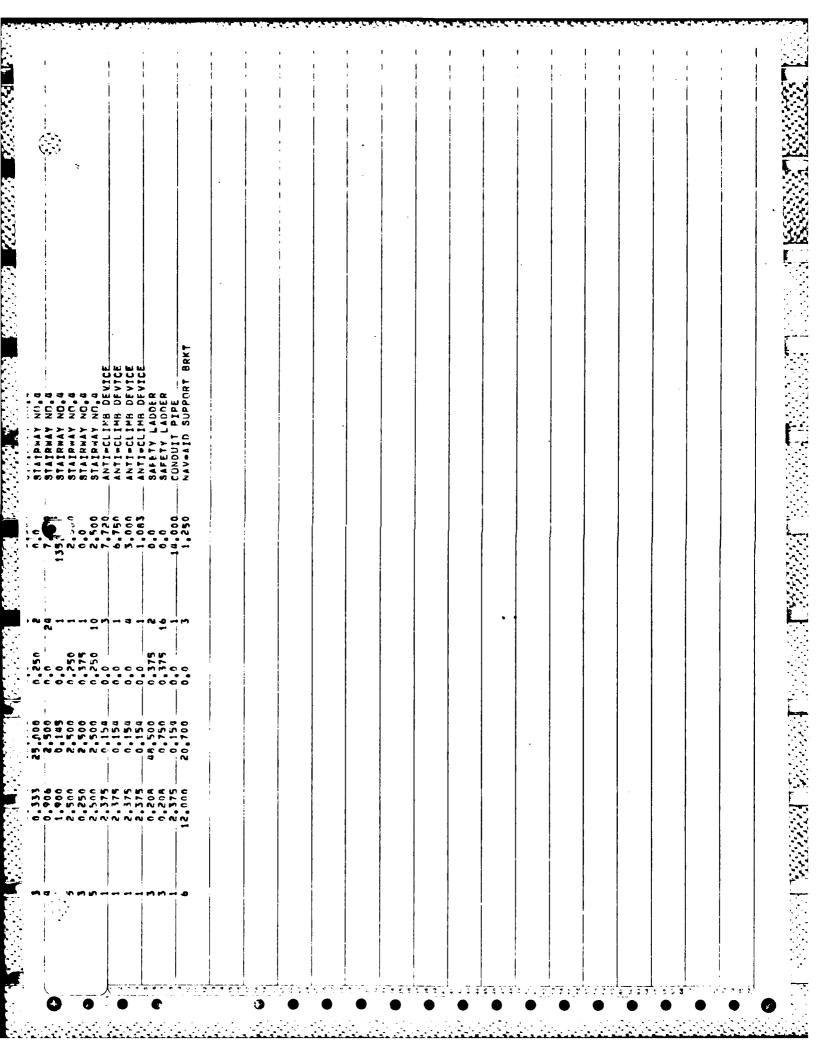
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APPENDIX B.6
MATERIAL LISTING

BILL OF MATERIALS & WETGHT FLRPLT .250THK.
FG POINT
FEG POINT
FG POINT
FG POINT FLRPLT .. 250 LEEG CCAN LEGG C CIACHRACE HORIZBRACE HORIZBRACE DIAGBRACE FLAPLTSUPPT 27-771-01 SHT U.S.NAVY ACMR PLATFORMS SUPERSTRUCTURE 500 LIST OF INPUT DATA --21.000





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33 X 3,00 X 0,250 & 3,53 84. 00 X 1,00 X 0,250 & 40.0 17 X X 0,250 & 41. 18 X 0,250 & 34. 19 X 0,250 & 34. 19 X 0,250 & 34. 19 X 0,250 & 34. 10 X 0,2	33 X 3,00 X 0,250 & 2,00 X 0,0	000 X X 000 X 000 X 000 X X 000 X 000 X X 000	00
17 X 2 LS X 0 2 SO 1 1 2 X 2 LS X 0 2 SO 1 2 SC 2 SO 2 SO 2 SO 3 SO 2 SO 3 SO 3 SO 3 SO	17 X 2 45	114 X 2 4 6 1 7 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	NA NA
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MEIGHT	TOTAL METGHT	21.0	TES. 8		•						
BILL OF MATERIALS &	TOTAL AREA	90°V	TOTAL HEIGHT OF PLATE	•							
UPERSTRUCTURE 27=771+01	QUANTITY	15									
U.S.NAVY ACMR PLATFORMS SUPERSTRUCTURE	NOTINAL DIMENSION	0,28 X 0,49 X 0,250 0,50 X 10,34 X 0,125									

U.S. uavy Acker PLATFORM SUMITY WEGER LEMETH TOTAL LEMETH TOTAL WITHT TOTAL WITH TOTAL WITHT TOT	NAVY ACHE PLATFORMS SUPERSTRUCTURE 27-771-01 BILL OF MATERIALS & METGHT TOTAL LENGTH TOTAL METGHT TOTAL METGHT TOTAL METGHT TOTAL METGHT TOTAL METGHT TOTAL METGHT TOTAL LENGTH TOTAL LENGTH TOTAL LENGTH TOTAL LENGTH TOTAL LENGTH TOTAL LENGTH TOTAL LENGTH TOTAL LENGTH TOTAL LENGTH TOTAL METGHT TOTAL METGHT TOTAL LENGTH LENGTH TOTAL LENGTH TOTAL LENGTH TOTAL LENGTH TOTAL LENGTH TOTAL LENGTH TOTAL LENGTH TOTAL LENGTH TOTAL LENGTH TOTAL LENGTH TOTAL LENGTH TOTAL LENGTH TOTAL LENGTH TOTAL LENGTH LENGT			*	<u>.</u>		
The color of the	The color of the	NAVY ACMR PLATEORMS	PERSTRUCTURE	27-771-01 BILL	OF MATERIALS & ME	1011	
0000 X 00,175 X 10,000 X 10,00	000 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	DMINAL DIERNOT IN X IN X IN		E ~	TOTAL LENGTH	TOTAL WEIGHT	
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2000 X 20	1000 X 000 X	TO A COC		0 4	0 10	2 P R	
100	000 x 0,375 2 1,83	0 X 000 H X 000	• • •	200	2 40	9.27	
0000 X 0,375 1 0,38	0000 X 0,775 7 0.58 1.33 1.33 1.33 1.33 1.33 1.33 1.33 1.3	O X 000 N X 000	~ N		3.67	15.7	
100 x 3,000 x 0,175	100 0	0 X 000 P X 000		1.33	1.33	17.0	
1000 x 0,775 1 1 0,000 x 0,775 1 1,23	1000 X 3,000 X 03,753	0 X 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	· • • •	6 O I	900		
100 x 3,000 x 0,250 4 1,33 1,20 x 2,500 x 2,500 x 0,250 4 2,50	1.33 1.33 5.33 1.67 1.67 1.67 1.67 1.67 1.67 1.67 1.67	O X OCC M X CCC	~	40.00	7.33	62.0	
1500 X 0.250 33 2.50 HOLES B 16	SOO X 2.500 X 0.250 33 2.50 E2.50 E2.50 TOTAL WEIGHT OF ANGLES * 16.50 E2.50 E		· 4 :	. M. C.	P III	200	
TOTAL WEIGHT OF ANGLES # 1608	TOTAL WEIGHT OF ANGLES # 1608	.500 x -2.500 x 0.5.	33	2,50	62.50	533,4	
				TOTAL	HEIGHT OF ANGLE	# # P	
					-		

GBATING					
NOMINAL DIMENSION	DUANTITY	UNIT WEIGHT	TOTAL AREA (- 90, FT)	TOTAL MEIGHT (-POUNDS-)	
× >		7,36	31.52	7.978	
	25	944	10.00	41.00 C	
×		7.36	2.01	0 7	
:		01	-TOTAL MEIGHT OF GRAFING-	ING-8	
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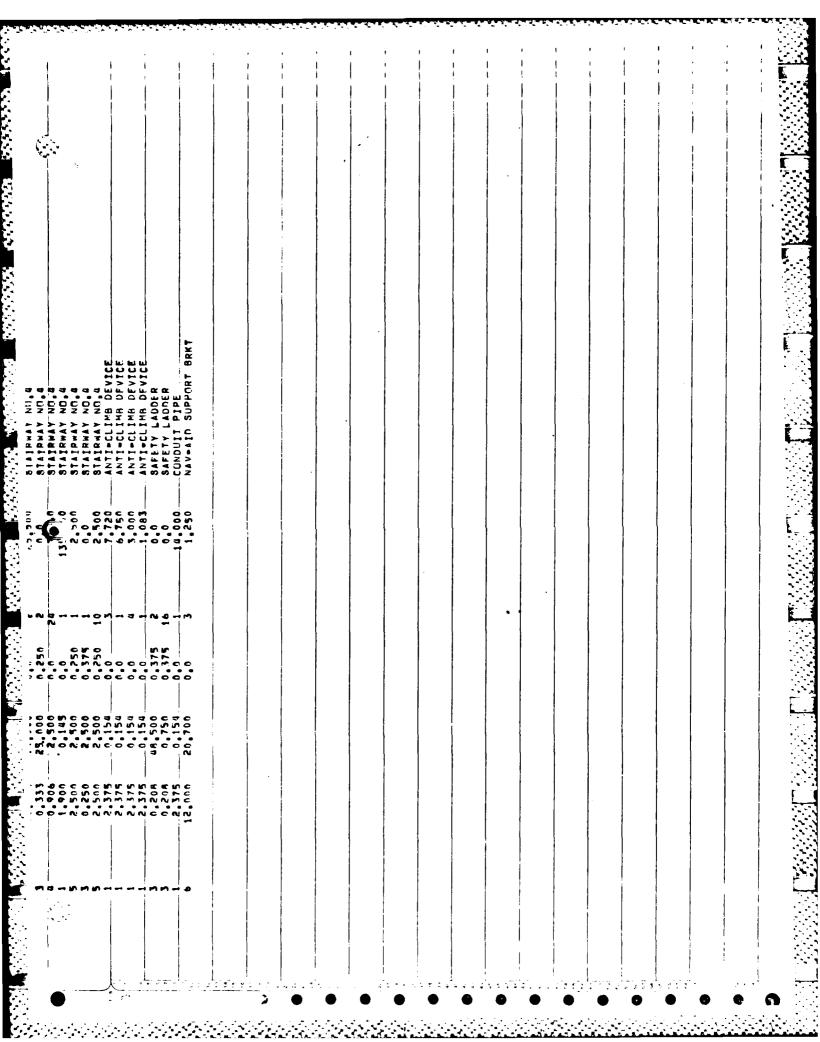
SUPERSTRUCTURE 27-771-01 BILL OF MATERIALS & WEIGHT	(PEET) TOTAL WEIGHT (PEET)	400	000	3,52	60.00	9,88	55.50 57.50	00.6	5.50 80.70	1213.00 3200.81		6.67	139-79 6089-60 65-04 1756-11				
BILL OF MATERIALS SUMMARY U.S.NAVY ACMR PLATFORMS SUPER	INAL DIME	 00 n.p. x 1.000	00 0 0 X 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	00 0.0 x 1.000	50 0.0. x 1.000 50 0.0. x 0.750	20 U.D. X 0.500	25 0.0 x 0.500 25 0.0 x 0.322	25 0.0 x 0.500	00 0 0 0 x 0 337	900 n.n. x 0.281 m7	:	W 21 X 73.0	H 18 X 50.00				

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	SUPERSTRUCTURE 27+771+01	AILL OF MATERIALS & MEIGHT	
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CHANNELS			
2 X - 25,000	9	3417,35	
10 x 15.50	14.43 11.00	730,30	
4 v G L E			
000 X 000 X 000	3,20	712.23	
000 X 000 X 0 0445	N . N .	101	
	10 00 00 00 00 00 00 00 00 00 00 00 00 0	26.08	
PSO THICKNESS	41,92	2130,71	
00000000000000000000000000000000000000	2	1566.07 1560.13 1260.33	
INS THICKNESS	5.51	13199,09 - (HSS2,7#)*	* 14"Chellered Bloom Plate
GRATING			
7,360 LBS PER SG FT 27	276,10	20.58	

COMMISSION CONTRACTOR CONTRACTOR

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	מודי"ם נודיונט מודי"ם נודיונט	7.0	7.4 LIP		IT.S TANDER	HT.S HANDRATE	S HANDRATE	HT.S HANDRATE-	4 4 ; z ? 2	42	ANTERNA MOUNT BATTERN BOX TIF DOEN	DEADEL SUPPO	SOLAR PAREL SUBDICATION	AR PANEL	SCIAR PAYEL SCUDDEN	AR PAVEL	AP PANEL 9	AR PANEL SUPP	AR PANEL S	"CZ >445 LS-	IRAAY	7444	TAIREAY NO	I R . A Y	1 - C 2 - M 4 C 1 + C 2 - M 4 C 1 + C 2 - M 4 C 1 + C 2 - M 4 C 1 + C 2 - M 4 C 1 + C 2 - C 2 - M 4 C 1 + C 2 - C	NA YAK	> > 4	DAY NO	TO CO. TO STATE OF THE CO.	STATES AND S	2 Z	N 24	ON AVE	ON AA	7 A 1 P	TATREAT NO.	TAIRAAY NO.	TAIRNAY NO.	TAIRNAT	M° 1. 2	TAIRARY NO	TAIDHAY N
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10.54	0 1.74	95.0	01.1	7.1.20	67°0 ×	1 0 48	163,10	71.0	25°0 0	7	00,50	3,70	6.00	3,00	10 m	3,00	CO M M M	1,00	3,00	00.50	0.50	0 53	2,50	05.0	7 2,45	5 3, 21	, F 0	325,20	0.28	3 35.23	16.36	2.50	n25_00	9.00		0.33	0 25,00	16,35	25.90	7.50	2.50	91.0
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MATERIAL LIBTING PROGRAM

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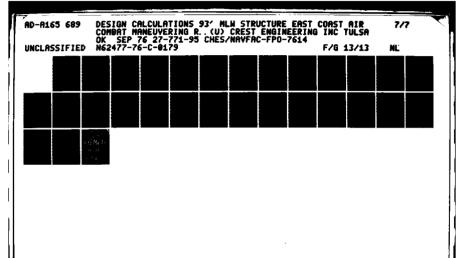
PIPE NDMINAL C IN X IN 1,900 0.0.	U.S.NAVY-ACMR. PLATFORMS—SUPERSTRUCTURE.	SUPERSTRUCTUR		-27 <i>0771</i> 001814 <mark>6-08-нат</mark> ғя146 8-8-к е16н 1	10H	
NDMIN (IN)						
1,000 1	NOMINAL DIMENSION	BUANTITY	MENBER LENGTH	TOTAL LENGTH	TOTAL MEIGHT	
	.O. X O.145 WT	-	53.00	53,00	100.2	
			TOTAL WES	TOTAL WEIGHT OF PIPE MEMBERS &	RS # A3325.1	
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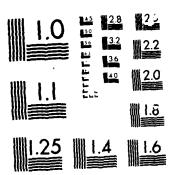
CONTRACTOR SOCIETY CONTRACTOR

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TITAL DIFERSION DUANTITY TY FT K IN 18 TOTAL WEIGHT OF PLATES = 233 TOTAL WEIGHT OF PLATES = 233	U. S. NAYY ACMR. PLAIFORMS S	1	
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MICROCOPY RESOLUTION TEST CHART

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NOWINAL DIMENSION	TOTAL LENGTH	TOTAL WEIGHT	
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× ×		7027 MG	
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2,750 N.D. X 1,000		1005,001	
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625 0.0 x 0.500	52	241012	
625 U.O. X 0.900		788.14	
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575 0.0° X 0.154		318, 59	1
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12. X27.0	65.04	1756.11	
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TOTAL WEIGHT 132752, 69 LBS	GRATING 7,360 LBS PER 30 FT 226,10 2052;10		.000 x 3.000 x 0.250 5.33 26.08 500 x 2.500 x 0.250 95.37 385.38	.000 x 3,000 x 0,175 73,20 712,25	J-9NF.	C 12 X 20,70 3474 77.62 77.62 C 10 X 15.30 51.00 780.30		6 x 14.50 250.67 3885.35	27-771-01 BILL		100 P	1-01 RILL OF MATERIALS & 3176-00 3845.35 70.30 716-2 3716-	SUPERSTRUCTURE 27-77 1 36,66 1 36,66 2 3,33 2 3,33 2 3,34 1 36,36 2 3,34 1 36,36 1 3	
7.360 LBS PER 30 FT276,102052;10		.050 THICKNESS	THICKNESS 41.02 2130.71 THICKNESS 109.30 THICKNESS 109.30 THICKNESS 109.30 THICKNESS 76.30 THICKNESS 76.30 THICKNESS 76.30 THICKNESS 76.30 THICKNESS 76.30	THICKNESS THICKNESS	0 x 3,000 x 0,375 73,20 197,26 0 x 3,000 x 0,375 20,00 0 x 3,000 x 0,375 20,00 0 x 3,000 x 0,250 5,33 20,00 0 x 2,000 x 0,250 95,37 20,00 0 x 2,500 x 0,250 95,37 20,00 0 x 2,500 x 0,250 95,37 20,00 0 x 2,500 x 0,250 20,37 20,00 0 x 2,500 x 0,250 20,30 20,00 0 x 2,500 x 0,250 20,30 20,00 0 x 2,500 x 0,250 20,30 20,00 0 x 2,500 x 0,250 20,30 20,00 0 x 2,500 x 0,250 20,30	THICKNESS THICKN	12 x 2000 x 15,70	THE KNESS 100 (150,00 1	ELS X 20,00	FLANT REFILES SUBMARY WE REAL SOLUTION SUPERSTRUCTURE 27-771-01 AILL OF MATERIALS & WEIGHT B K 28,00 E 12 K 29,	1	(#5.2821) - (#582.74 70.17	186,36(1025F 7 15,51	125
125 THICKNESS 15.51 70.17 RATING 7.360 LBS PER 50 FT 276,10 2052,10	12.51 THICKNESS 15.51 79.17			0 x 2,500 x 0,250 5,33 26,08 0 x 2,500 x 0,250 95,37 3A5,38	0 x 3,000 x 0,379 73,20 712,23 197,26 0 x 3,000 x 0,379 23,33 197,26 0 287,11 20 0 x 3,000 x 0,250 5,33 26,08 0 x 2,500 x 0,250 95,37 3.5,36	0 x 3,000 x 0,375 73,20 712,23 0 x 3,000 x 0,375 25,33 197,26 0 x 3,000 x 0,250 5,33 26,06 0 x 2,500 x 0,250 95,37 385,38	12 x 20,00 12 x 20,70 12 x 20,70 13 x 20,70 14 x 15,30 15 x 20,00 16 x 3,000 x 0,379 17 x 20 18 x 2,000 x 0,379 18 x 3,000 x 0,379 18 x		ELS ELS A 18,50 A 18,50 B 20,67 B 34,7 135 C 12 X 28,00 S 34,7 135 C 12 X 28,00 S 34,7 135 C 12 X 28,00 S 3,35 C 13 X 28,00 S 3,35 C 14 X 28,00 S 3,35 C 15 X 28,00 S 3,35 C 15 X 28,00 S 3,35 C 15 X 28,00 S 3,35 C 15 X 28,00 C 18,00 C 18,000	ПР MATERIALS SUMMARY LALY ACHR PLATFORMS SUPERSTRUCTURE 27-771-01 MILL OF MATERIALS & MEIGHT M	1/4"Chubard 320m 100.82	2139,71 8463,16 1046,67 1569,18 1260,09 1260,09	41,92 109,30 34,18 76,86 82,34	000 000 000 000 000 000 000 000 000 00

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7.500 OHT.	. 440	7.000	7.000	.740	6000	0.970	150	750	3.580	770	1,350	7.567	225	3.083	3.250	6.50	7.000		.500	0.0	222	8.420	.360	140	7.750	1.583	727		9			e. (000	500	0	000	.250	0.	٠,					
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44.000	5.50	7.50	7.50	5.50	7.00		0000	000		2.75	2.75		200	0.0	00.0	•		1.25	8	52.0	5.4	62	5	0 5	0.75	0.75	200	1.55	55.		8	2,5		.75	2 .	. 20	.50	\$2.	6 4	33	63	99.	4 7	č

ANNEXA LAGRANGE MANAGEMENT MANAGEMENT

. U.S.NAVY ACHR PLATFURM. 93. FT.MLN. JACKET 27A1711=01. BILL_DR. MATERIALS B. MEIGHT.

PIPE

	7. 1970	80.0	04.1	16.1	32.6		0.04	82.1	73.3	0.48	2.5				61.7			11.5	76.5	0.07	54.0	•	3079.6	-		N. 40	•	5 6	1 0	2 Y Z	
TOTAL MEIGHT	76.	308	110	157	551	247 24	950	15	15	~1	224	169	187	23	22	1803	218	85	901	93	19	20	C M	56	2	Ē.,					•
TOTAL LENGTH	22.50	42.00	16.00	65.34		25,25	89.44	10,25	8,25	00 8	173,28	131.04	144.78	05.61	18.50	14,75	212,70	80.91	147.96	65.00	124.05	103	4.00	75	U	00.4	2001	1 P P P P	18.75	11.00	
MEMBER LENGTH	7.50	7,00	9	21.72			38,15	44.6	2,78	2.67	57,76	43,68	24,13	3,25	3.08	97.2	35,45	26,97	6. M. O. O. O. O. O. O. O. O. O. O. O. O. O.	C(5.0)	41.55	53,77		61.38	C	e c	67. 41	. a	1.25	00 4	
DUANTITY	97 1 98	•	14J \$		~	· •	•	₩.	-	•	_		•	•	6	•	• V		.	m :		n.	- 1		n r	n :	-		1	=	12
NORTH POINT ON (IN X IX)	×× °C C	7.500 n.p. x 1.500	000 T.D. X 1.250	A CO CO CO CO CO CO CO CO CO CO CO CO CO		4.000 D.D. X. 0.875	0,000 0,0 x 1,12	0.000 U.D. x 0.750	0.000 D.D. X 9.7	0.000 U.D. X 0.750	369 C. P. X G.	0.000 n.o. x 0.6	0.000 n.o. x o.6	6.000 n.0. x 0.750	00 X "C"U 000	300 P.D. X 0.750	200 N. O. 624	100 E.D. X 0.025	300 C X 200 C	200 T 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	130 L.U. A 0.375	2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		THE COLUMN TO THE			25 U.D. X 0.500	255 P.D. X 0.280	500 0.0 x 0.216	500 M.D. X 0.21	175 C.D. X O.

TOTAL WEIGHT OF PIPE MEMBERS .

				*;
U.S.NAVY ACHR PLATFORM 65 FT 41 4	t-Jacket-27e771e01-BILL-Op-Material S-	9-8-KRIGE4		
NOMINAL DIMENSION GUANTI	TV TOTAL ARE	A TOTAL METGHT	L HOIL	
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10 mm			4910.8	
			13 F B B B B B B B B B B B B B B B B B B	
00 x 2 a6 x 0 625			99199	
Note of the state			1130.1	
50 x 1,50 x 0,500			1225,0	
25 X 1,25 X 0,375			, t	
25 X 2 1,00 X 0,175			15,3	
44 X 0.44 X 0.250				
	TOTAL WEIGHT OF	PLATES # 2	5000.2	
		•		

W Z X X X X X X X X X X X X X X X X X X	ANGLE			1		
6.000 x 0.375 6 4.00 Z24.00 Z24.00 Z24.00 TOTAL WEIGHT OF ANGLES = Z294.6 TOTAL WEIGHT OF ANGLES = S294.6	INAL DIMENSI	QUANTITY	MEMBER LENGTH	TOTAL LENGTH	TOTAL MEIGHT	
MEIGHT = 354497,6	4,000 x 0,015	•	00.4	24.00	294.8	
354407.6			1074	MEIGHT OF ANGLES	•	
354407,6						
					•	
4						

7 - 3 - 3 - 3 - 3 - 4 - 4

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93 FT MLW JACKET 27-771-01 BILL OF MATERIALS & WEIGHT TOTAL MEIGHT (POUND) TOTAL LENGTH (FEET) BILL OF MATERIALS SUMMARY U.S.NAVY ACMP PLATFORM NOWINAL DIMENSION 3dld

6	771-01 MILL OF MATERIALS & METCHT			•	10A 14B	4010,79			63.16	17.33		1444,67					
	PT 464 346KET 274		00*82	•		96,21	40°44	318.12	11.77	1 d 70		196,28	ME I GHI				
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LIST OF INPUT DATA ... U.S.NAVY ACHR PLATFORMS JACKET BOAT LANDING 27-771-01 BILL OF MATERIALS

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U.S.NAVY ACHR PLATFORMS JACKET BOAT LANDING 27-771-01 BILL UF HATERIALS

U.S.NAVY ACHR PLATFORMS JACKET BOAT LANDING 27-771-01 BILL OF HATERIALS

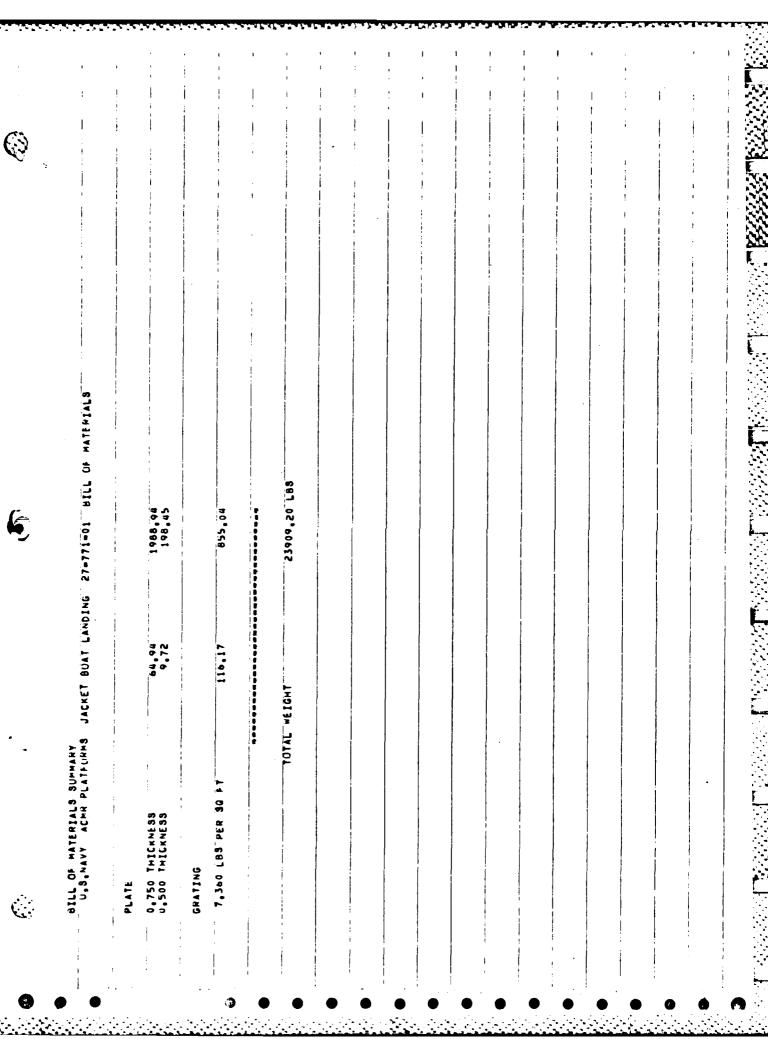
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_	~	00 %	00.4	429.2
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_	~	1,00	00°2	214.6
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	3	21,75	87,00	3778.3
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	•	4.33	00.45	1129,1
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	7	12,85	25,67	734,1
	7	10.44	41,76	1194.3
-		9,75	9,75	278,9
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↓	HILL OF MATERIALS	TOTAL LENGTH	2,50 8,50 7,50			TOTAL WEIGHT							
(ANDING 27-771-01	HEMBER LENGTH	1,625 2,25 3,75	HT OF CHANNELS					·				
	S JACKET BOAT LANDIN	CCANTITY	~~~	TOTAL WEIGHT OF									
	U.S. NAVY ACAR PLATFORMS	NOMINAL DIMENSION	C 8 X 80.70 C 6 X 8.20 C 6 X 8.20										

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CONTRACTOR SECURIOR CONTRACTOR

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U.S.NAVY ACMR PLATFURMS BARGE FENDERS 274771401 BILL OF MATERIALS & MEIGHT

13425.5 TOTAL MEIGHT TOTAL WEIGHT OF PLATES # U.S.NAVY ACMR PLATFUHMS BARGE FENDERS 27-771-01 BILL OF MATERIALS & MEIGHT TUTAL WEIGHT TUTAL AREA DUANTITY PURINAL DIRENGION PLATE

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BILL UF MATERIALS SUMMARY
U.S.NAVY ACMM PLATFURMS BARGE FENDERS 274771401 BILL OF MATERIALS & MEIGHT

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U.S.NAVY ACMM PLATFUMMS BARGE FENDERS 27-771-01 BILL OF MATERI.
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TOTAL

JACKET PILING SITE 2 27-771-01 BILL OF MATERIALS SECTION
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U.S.NAYY ACHR PLATFORM ..JACKET_PILING SITE_2...27#771#01 BILL OF MATERIALS

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SASAL KARAGASA

BILL OF HATERIALS SURMARY LAC	JACKET PILING SITE 2	27-771-01 RILL OF MATERIALS	
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